



VERANDA AT PLEASANT VALLEY

TRAFFIC IMPACT STUDY

GRESHAM, OREGON



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DATE:
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EXECUTIVE SUMMARY

1. A property located on the east side of SE 190th Drive south of SE Richey Road in Gresham, Oregon is proposed for development with 183 single-family homes. The site will take access via two street connections to SE 190th Drive.
2. The subject property is projected to generate 135 new site trips during the morning peak hour, 181 trips during the evening peak hour, and 1,728 new daily site trips.
3. Based on the operational analysis, all study intersections will either operate acceptably per City of Gresham standards or operate with reduced delays and increased capacity as compared to background conditions upon completion of the proposed development provided that a southbound right-turn lane is provided for the intersection of SE 190th Drive at SE Richey Road, and a westbound left-turn lane is provided for the intersection of SE Foster Road at SE Richey Road. No other mitigations are recommended in conjunction with the proposed development.
4. Based on a queuing simulation model, the projected 95th percentile queue lengths for the proposed new turn lanes were 105 feet for the southbound right-turn lane serving SE 190th Drive at SE Richey Road, and 161 feet for the westbound left-turn lane serving SE Foster Road at SE Richey Road. Accordingly, the turn lanes provided should accommodate these minimum storage requirements.
5. Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of SE 190th Drive at SW Butler Road has a calculated crash rate of 1.34 crashes per million entering vehicle, consisting primarily of angle collisions. However, the traffic signal currently being installed is expected to significantly reduce the frequency of these crashes. Based on the crash data analysis, no other safety mitigations are recommended in conjunction with the proposed development.
6. Intersection sight distance was evaluated for the two new site access intersections on SE 190th Drive. The proposed site access locations are 857 feet and 1,173 feet south of the centerline of Richey Road. From these locations, adequate sight distance is projected to be available for safe and efficient operation of the intersections.



PROJECT DESCRIPTION & LOCATION

INTRODUCTION

The proposed development is located on the east side of SE 190th Drive south of SE Richey Road in Gresham, Oregon. The site is proposed to be developed with 183 single-family dwellings. Access will be taken via two new streets intersecting SE 190th Drive.

This report addresses the impacts of the proposed development on the surrounding street system. Based on the City of Gresham's code requirements, an operational and safety analysis is required for all intersections projected to experience an increase of 5 percent or more of any approach lane capacity during either the morning or evening peak hours. Accordingly, the analysis scope includes the two proposed site access intersections on SE 190th Drive and the existing public intersections of:

- SE Powell Boulevard at SE 182nd Avenue;
- SW Pleasant View Drive/SE 190th Drive at SE Giese Road/SW Butler Road;
- SE 190th Drive at SE Richey Road; and
- SE Foster Road at SE Richey Road.

The purpose of this analysis is to determine whether the surrounding transportation system is capable of safely and efficiently supporting the proposed use and to identify any necessary improvements and mitigations.

SITE LOCATION AND STUDY AREA DESCRIPTION

The project site totals approximately 38.9 acres and is developed with one single-family home, which is currently vacant. The property is bordered by the Brookside at Pleasant Valley development to the north, and by existing low-density residential uses as well as agricultural and forested lands to the west, east and south.

A description of the streets within the study area is provided in Table 1 below.

Table 1 - Study Area Roadway Descriptions

Roadway	Functional Classification	Travel Lanes	Bike Lanes	Sidewalks	Speed (mph)
SE Powell Boulevard	Arterial / District Hwy	5	Yes	Yes	40
SE 182nd Avenue	Arterial	5	Yes	Yes	35
SW Pleasant View Drive	Arterial	2 - 3	Partial	Partial	40
SE Giese Road	Minor Arterial	2	No	No	40
SW Butler Road	Minor Arterial	2	No	Partial	25
SE 190th Drive	Arterial	2	No	No	40
SE Richey Road	Collector	2	No	No	35* - 45
SE Foster Road	Minor Arterial	2	No	No	45*

*20 mph within designated school zone between 7:00 AM and 5:00 PM on school days.



EXISTING CONDITIONS

The intersection of SE Powell Boulevard (US Highway 26) at SE 182nd Avenue/SW Highland Drive is controlled by a traffic signal. The eastbound, westbound, and northbound approaches each have a left-turn lane operating with protected phasing, a dedicated through lane, a shared through/right lane, and a bike lane to the right of the motor vehicle lanes. The southbound approach has a left-turn lane with protected phasing, two dedicated through lanes, a right-turn lane, and a bike lane between the through and right-turn lanes. Marked crosswalks with pedestrian signals are in place crossing all four legs of the intersection.

The intersection of SW Pleasant View Drive/SE 190th Drive at SE Giese Road/SW Butler Road currently operates under all-way stop control, but a traffic signal is currently being installed. The northbound and westbound approaches each have a shared left/through lane and a dedicated right-turn lane. The southbound approach has a left-turn lane and a shared through/right lane. The eastbound approach has a single, shared lane for all turning movements.

The intersection of SE 190th Drive at SE Richey Road is a T-intersection controlled operating under all-way stop control. Each approach has a single, shared lane for all turning movements.

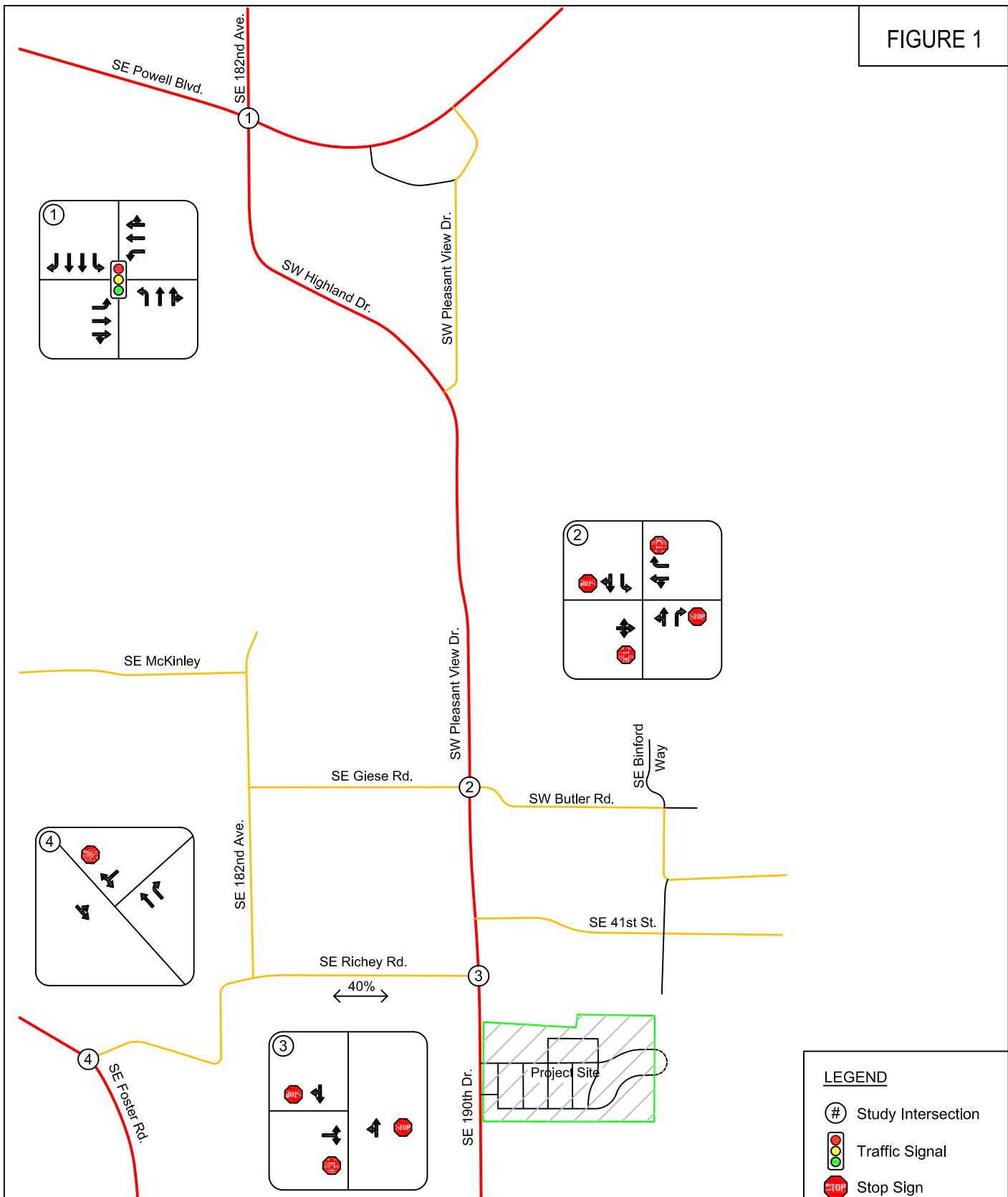
The intersection of SE Foster Road at SE Richey Road is a T-intersection controlled by a stop sign on the westbound Richey Road approach. Through traffic traveling along SE Foster Road does not stop. The southeast-bound and westbound approaches each have a single, shared lane for all turning movements. The northwest-bound approach has a dedicated through lane and a channelized right-turn lane operating under yield control.

A vicinity map displaying the project site, vicinity streets, and the study intersections including lane configurations is provided in Figure 1 on page 6.

TRAFFIC COUNT DATA

SE Richey Road is currently under construction and is closed to through traffic. Accordingly, count data for the study intersections was drawn from prior studies conducted in the site vicinity. The prior traffic counts were collected at the study area intersections on Tuesday May 7, 2019 from 7:00 to 9:00 AM and 4:00 to 6:00 PM. Those volumes were adjusted to account for two years of growth in traffic to determine the current year 2021 traffic volumes. Figures 2 and 3 on pages 7 and 8 show the existing 2021 traffic volumes for the morning and evening peak hours at the study intersections.

FIGURE 1



VICINITY MAP
Study Intersections
Lane Configurations and Traffic Control

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FIGURE 2

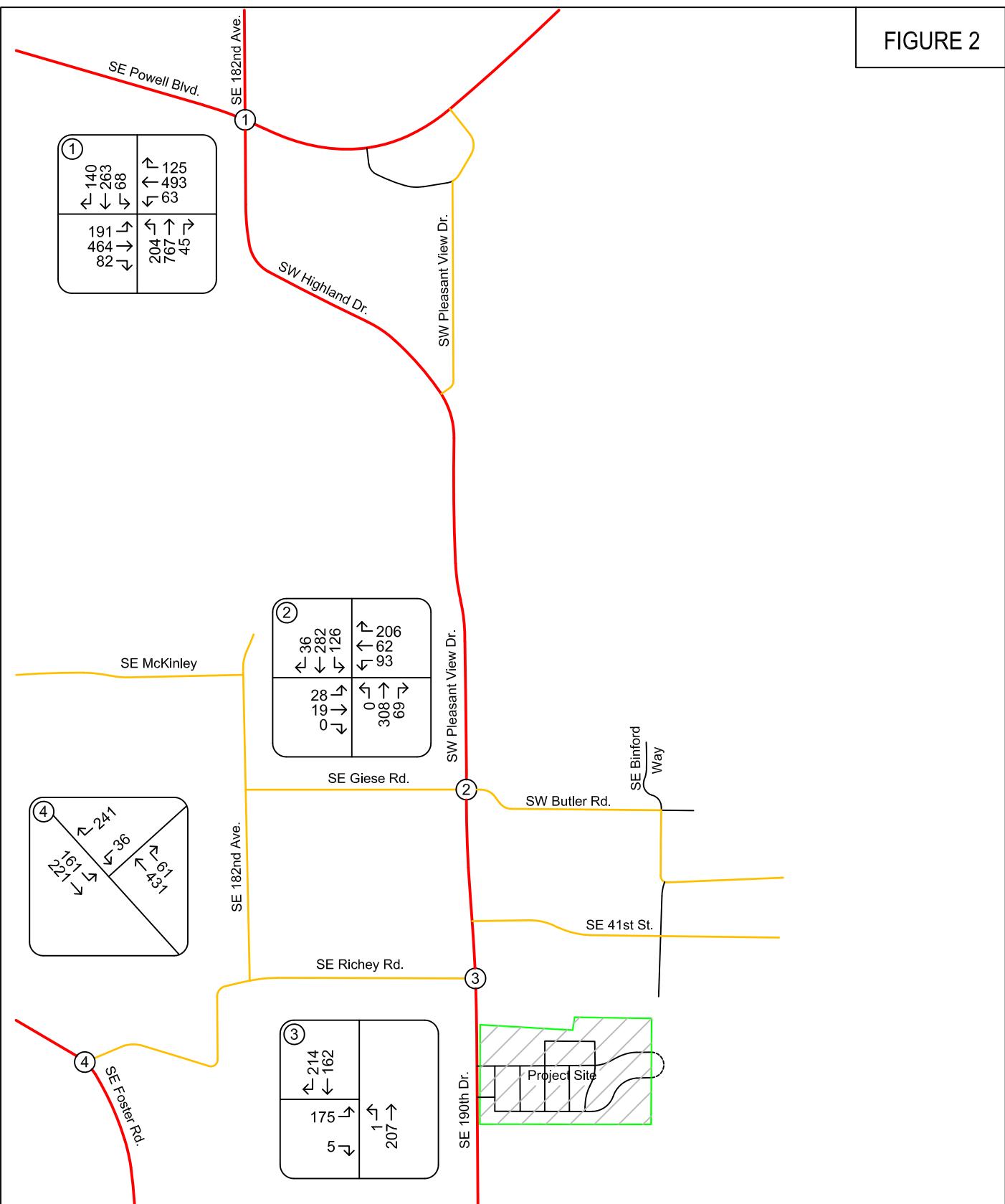
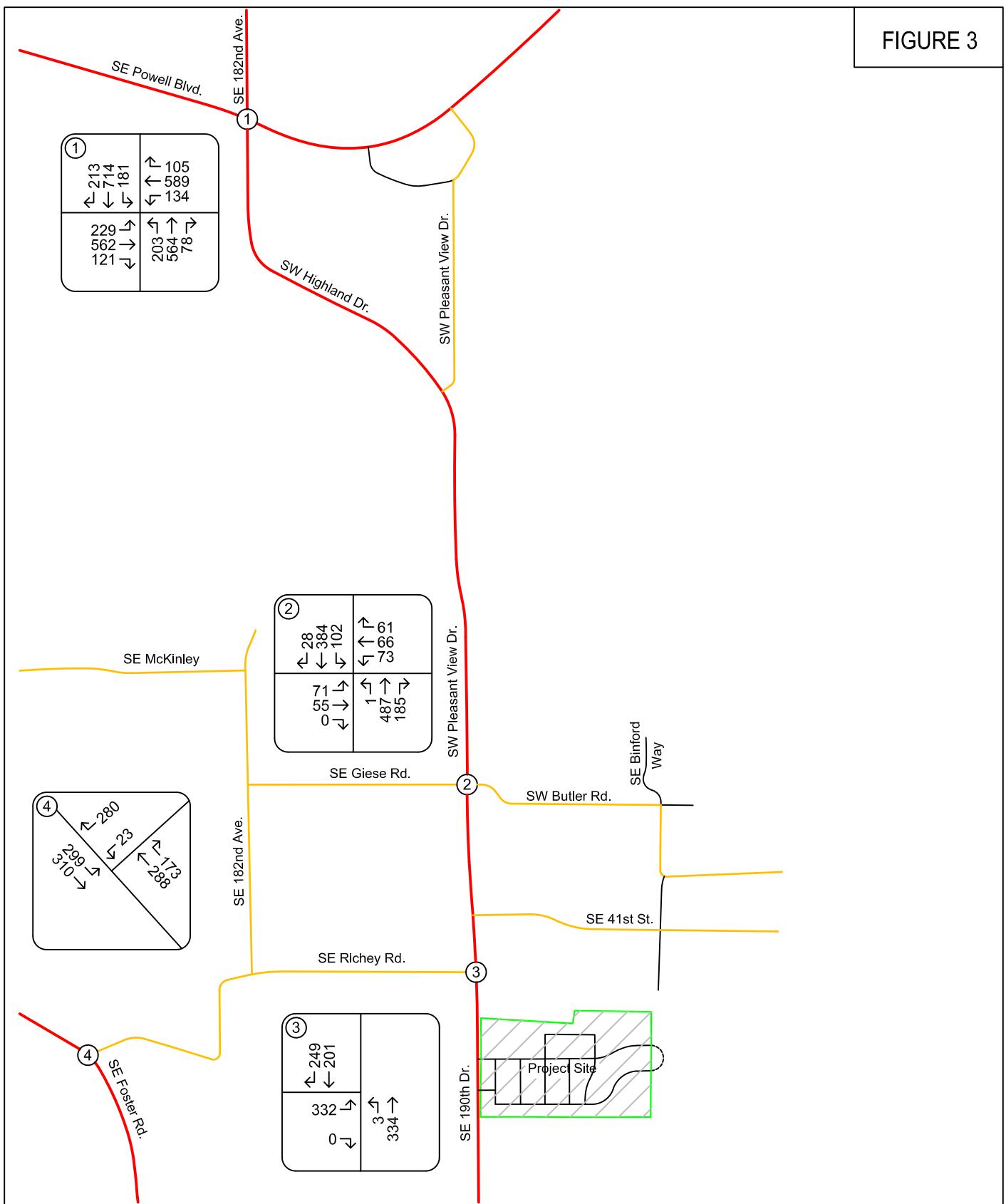


FIGURE 3





OPERATIONAL ANALYSIS

An operational analysis was conducted for the study intersections using Synchro 10 software, with outputs calculated based on the *HIGHWAY CAPACITY MANUAL, 6th Edition*. The analysis was conducted for the weekday morning and evening peak hours.

The purpose of the existing conditions analysis is to establish how the study area intersections operate currently and allow for calibration of the operational analysis if required.

The results of the operational analysis are reported based on delay, Level of Service (LOS), and volume-to-capacity ratio (v/c). Delays are reported in seconds. Level of service is reported as a letter grade and can range from A to F, with level of service A representing nearly free-flow conditions and level of service F representing high delays and severe congestion. A report of level of service D generally indicates moderately high but tolerable delays, and typically occurs prior to reaching intersection capacity. For unsignalized intersections, the v/c represents the portion of the available intersection capacity that is being utilized on the worst intersection approach. For signalized intersections, it indicates the portion of the overall intersection's capacity that is being used. A v/c ratio of 1.0 would indicate that the intersection is operating at capacity.

The City of Gresham requires that signalized intersections operate with a v/c ratio of 0.90 or less and at level of service D or better during the peak hours. Individual movements at signalized intersections must operate at level of service E or better and with a v/c ratio of 1.0 or less. Unsignalized intersections are required to operate at level of service E or better.

A summary of the existing conditions operational analysis is provided in Table 2 below. For the unsignalized intersections the reported delays and levels-of-service represent the approach lane which experiences the highest delays. The reported v/c ratios represent the highest ratio for any approach lane. For the signalized intersection of SE Powell Boulevard at SE 182nd Avenue the reported delays, levels-of-service and v/c ratios represent the operation of the overall intersection.

Based on the analysis, most study intersections are currently operating acceptably per City of Gresham standards. The intersection of SE 190th Drive at SW Butler Road is currently operating at level of service F and with a v/c ratio of 0.93 during the evening peak hour; however, it is currently being signalized, which will restore acceptable operation. Detailed capacity analysis worksheets are provided in the technical appendix.

Table 2 - Operational Analysis Summary: 2021 Existing Conditions

Intersection	AM Peak Hour			PM Peak Hour		
	Delay	LOS	v/c*	Delay	LOS	v/c*
SE Powell Blvd. at SE 182nd Ave.	35.9	D	0.82	48.9	D	0.78
SE 190th Dr. at SE Giese Rd./SW Butler Rd.	21.2	C	0.64	50.7	F	0.93
SE 190th Dr. at SE Richey Rd.	12.3	B	0.52	19.1	C	0.69
SE Foster Rd. at SE Richey Rd.	17.2	C	0.53	13.1	B	0.42



SITE TRIPS

Proposed Development

The proposed new development will consist of 183 single-family dwellings. To estimate the number of trips that will be generated by the proposed development, trip rates from the *TRIP GENERATION MANUAL, 10th EDITION* were used. Data from land-use code 210, *Single-Family Detached* were used. The trip estimates are based on the number of dwelling units.

A summary of the trip generation calculations is provided in Table 3 below. Detailed trip generation worksheets are also included in the technical appendix.

Table 3 - Proposed Development Trip Generation Summary

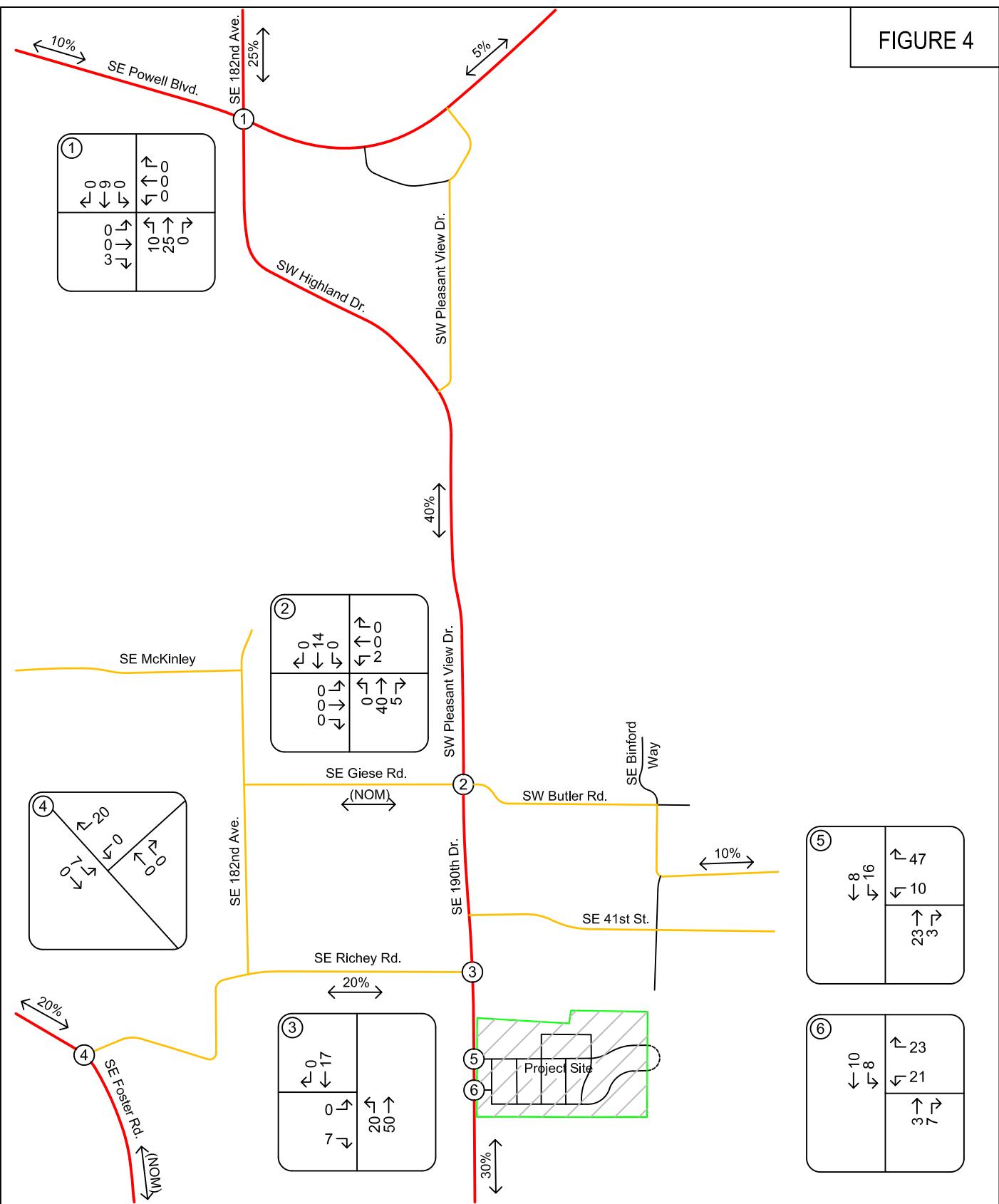
	AM Peak Hour			PM Peak Hour			Daily Total
	In	Out	Total	In	Out	Total	
183 Single-Family Homes	34	101	135	114	67	181	1728

TRIP DISTRIBUTION

The directional distribution of site trips to and from the project site was estimated based the existing travel patterns in the site vicinity, as well as the locations of likely trip destinations and major transportation routes. Overall, 30 percent of the anticipated site trips are projected to travel to and from the west on SE Foster Road, 20 percent will travel to and from the south on SE 190th Drive, 10 percent will travel to and from the east on SW Butler Road, 10 percent will travel to and from the west on SE Powell Boulevard, 25 percent will travel to and from the north on SE 182nd Avenue, and 5 percent will travel to and from the east on SE Powell Boulevard.

The trip distribution percentages and trip assignment for the proposed development are shown in Figures 4 and 5 on pages 11 and 12.

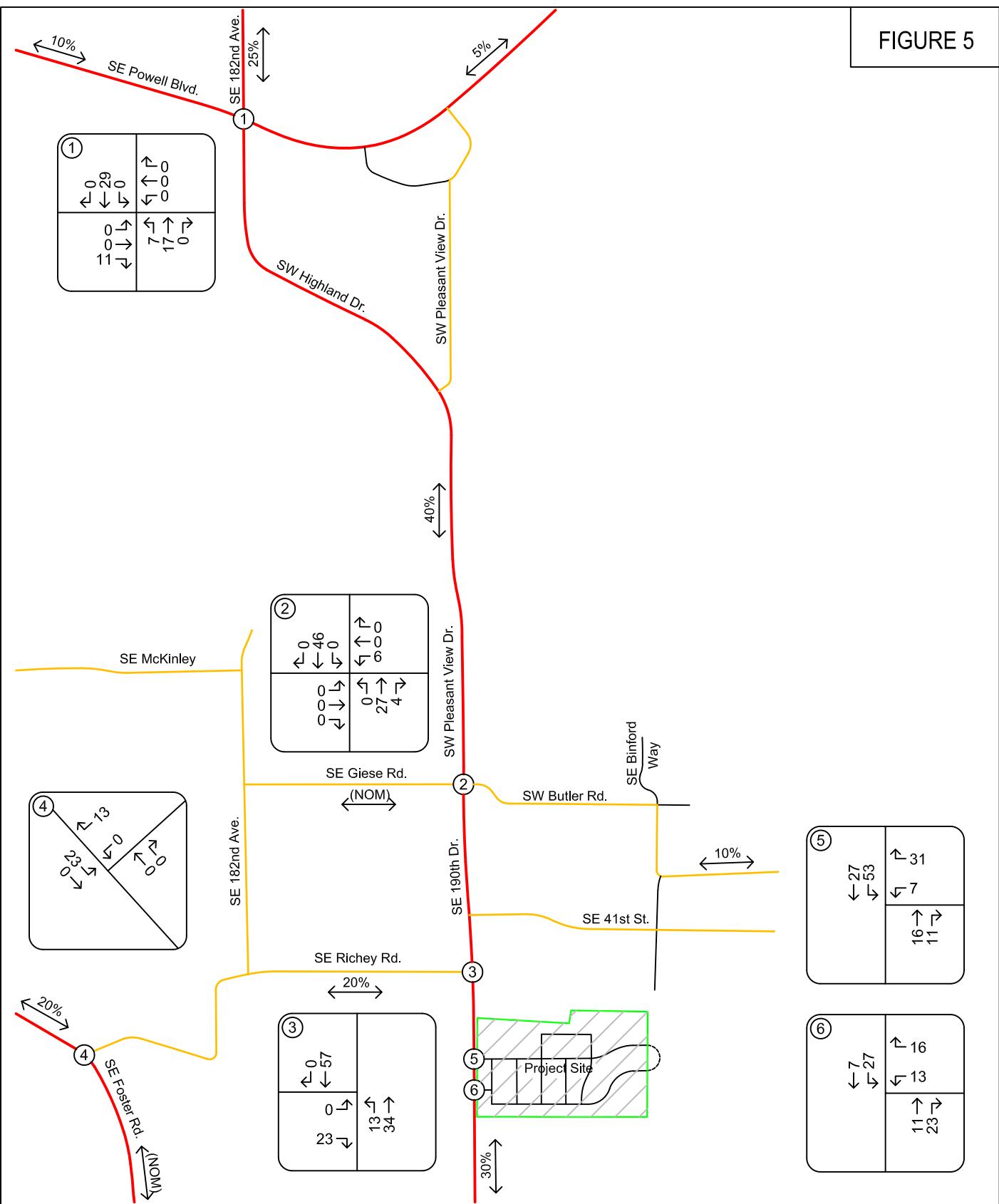
FIGURE 4



TRAFFIC VOLUMES
Proposed Development - Site Trips
Morning Peak Hour

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FIGURE 5



TRAFFIC VOLUMES
Proposed Development - Site Trips
Evening Peak Hour

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FUTURE CONDITIONS ANALYSIS

BACKGROUND VOLUMES

In order to determine the expected impact of site trips on the study area intersections, it is necessary to compare traffic conditions both with and without the addition of the projected traffic from the proposed development. Since the proposed use cannot be constructed and occupied immediately, the comparison is made for future traffic conditions at the time of anticipated project completion. It is expected that the proposed use will be completed and occupied within three years. Accordingly, the analysis was conducted for year 2024 traffic conditions.

To generate the year 2024 background traffic volumes, turning movement data was taken from the prior Brookside at Pleasant Valley (Phases 4, 5 and 6) traffic impact study. Specifically, data was taken from Figures 8 and 9, which showed the year 2022 “Background Plus Site Trips Conditions”. Accordingly, these volumes include in-process trips from the Sunset Village, Sycamore Vista, Del Boca Vista, Brookside Phases 3, 4, 5, and 6, Big Oak, and Homestead developments.

Since the data utilized represented year 2022 traffic conditions, it was necessary to adjust the volumes to account for two additional years of growth to generate the year 2024 background traffic volumes. In accordance with the procedures described in ODOT’s Analysis Procedures Manual and based on data from ODOT’s Future Volume Tables, a linear growth rate of 1.002 was calculated for traffic volumes on SE Powell Boulevard in the site vicinity. Growth rates for all other turning movements were conservatively calculated based on the average growth rate for the Portland Metro area over the most recent three years, which is in excess of the average growth rate for the City of Gresham. This equated to a background growth rate of 1.27 percent per year. The growth rates were applied over a period of two years.

Note that since the year 2022 traffic volumes projected in the prior report assumed completion of all relevant in-process projects (and the Brookside project from which the data was taken), the projected year 2024 traffic volumes also conservatively assume that all in-process developments will be complete by 2024.

Figures 6 and 7 on pages 14 and 15 show the projected year 2024 background traffic volumes at the study intersections during the morning and evening peak hours.

BACKGROUND VOLUMES PLUS SITE TRIPS

Peak hour trips calculated to be generated by the proposed development were added to the projected year 2024 background traffic volumes to obtain the year 2024 total traffic volumes following completion of the proposed residential development.

Figures 8 and 9 on pages 16 and 17 show the projected year 2024 peak hour volumes including background growth and site trips from the proposed development.

FIGURE 6

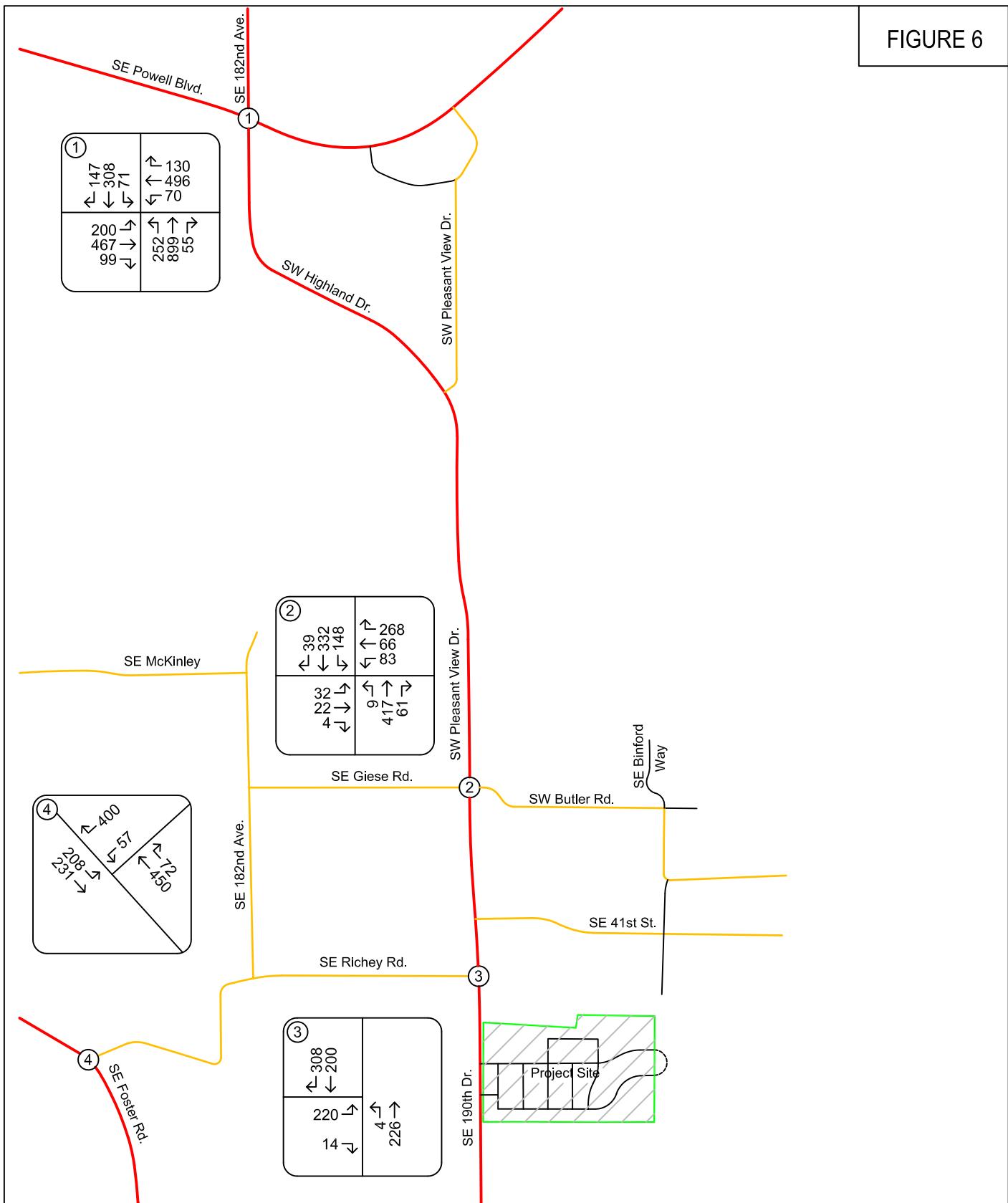


FIGURE 7

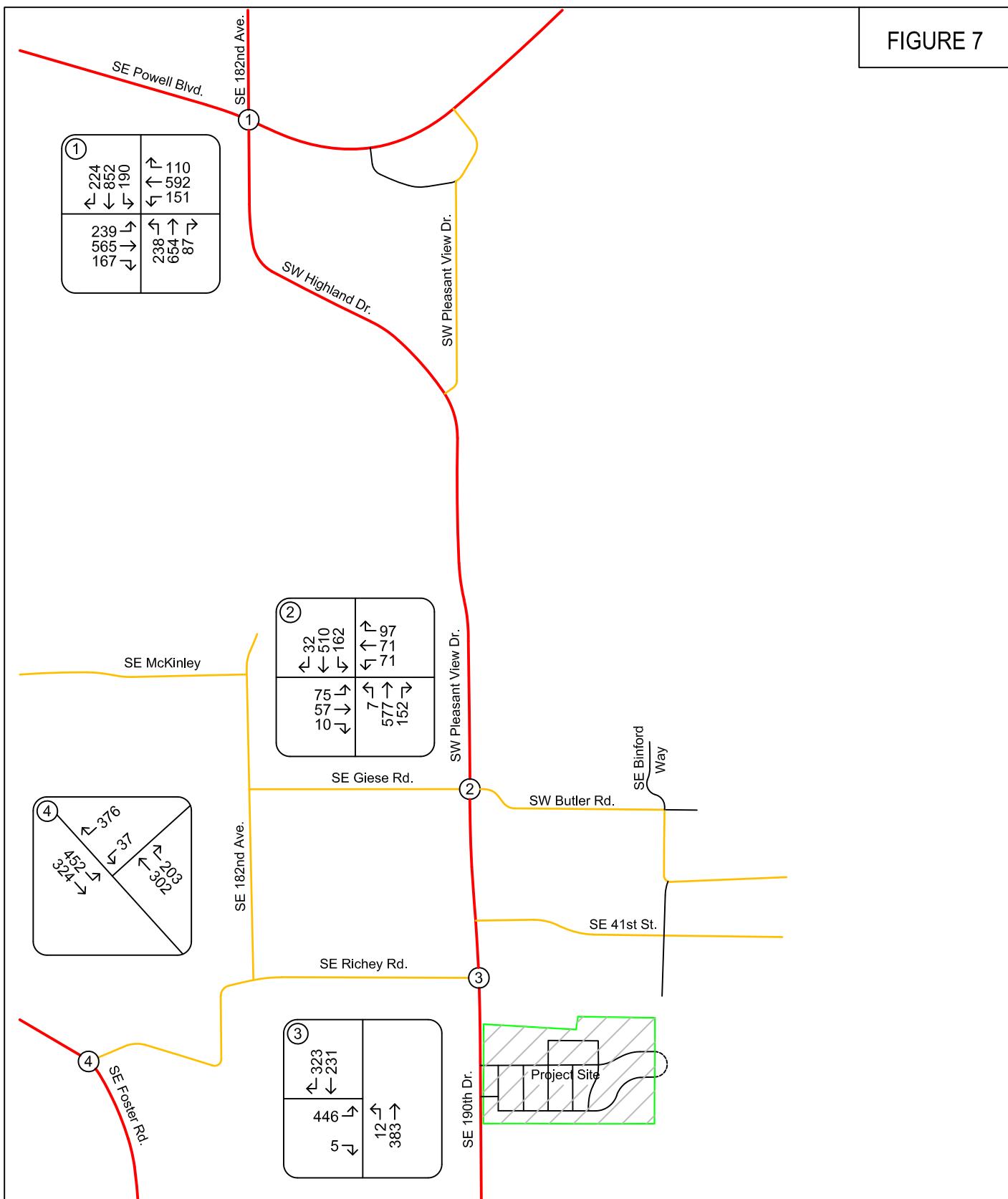


FIGURE 8

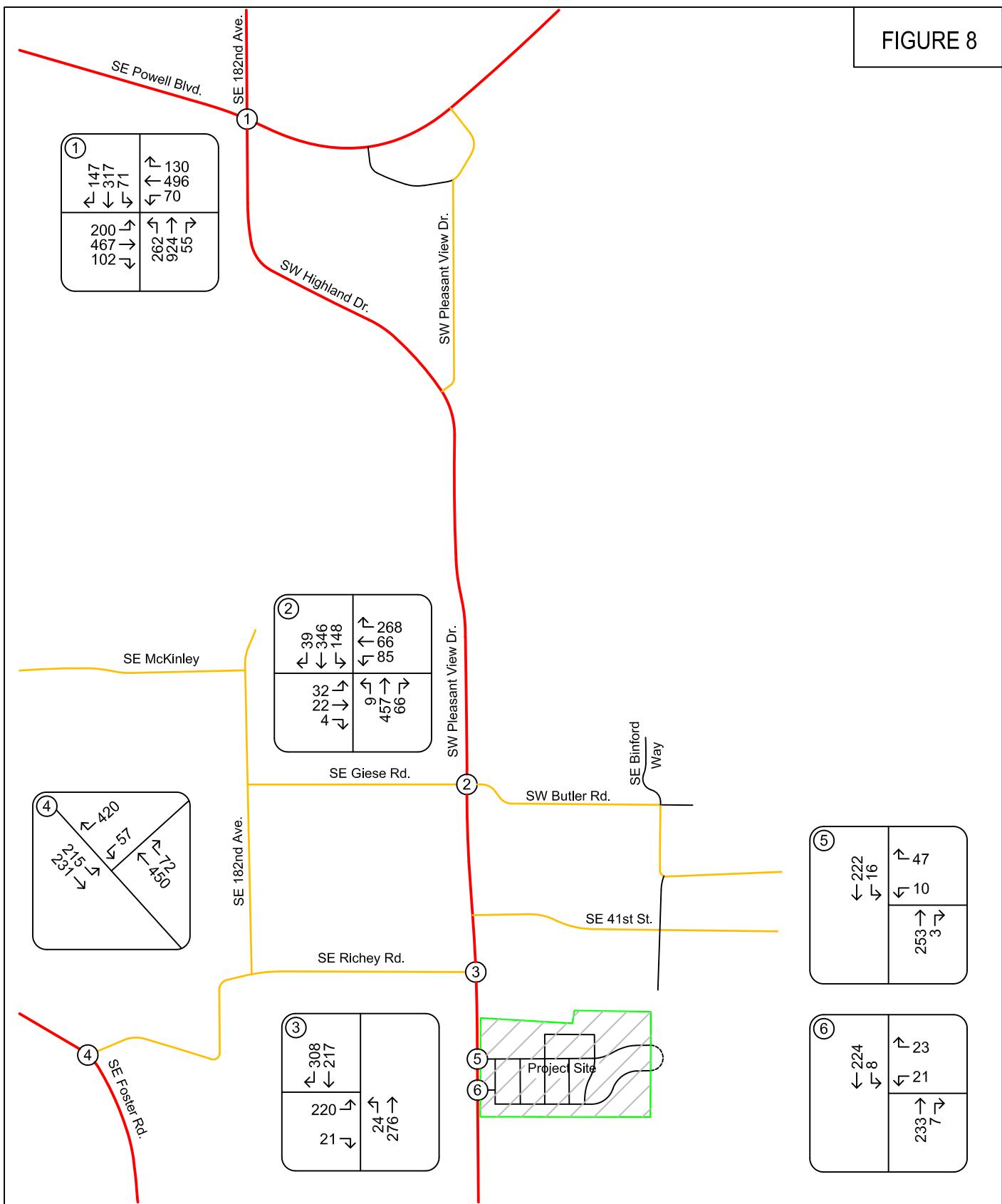
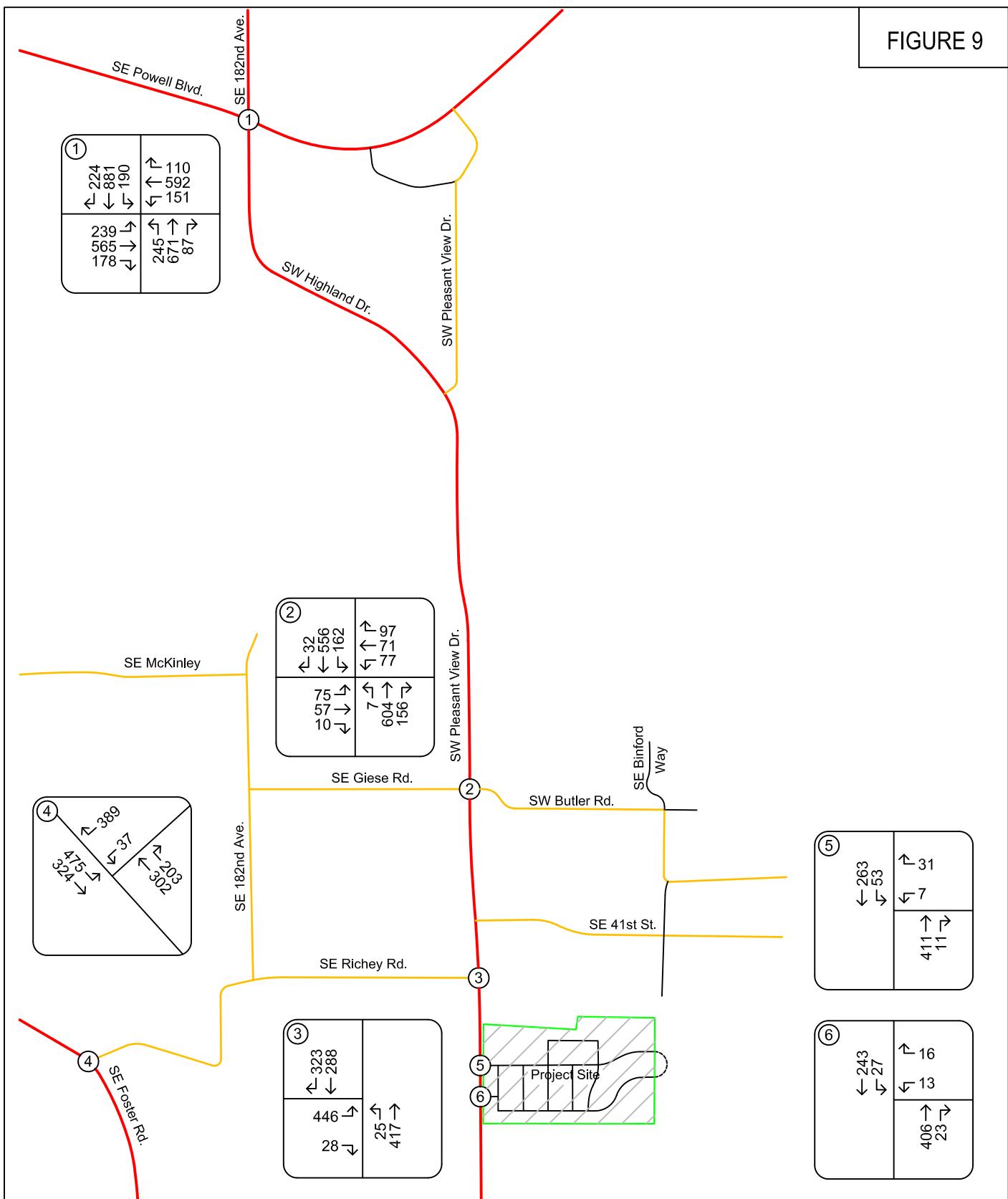


FIGURE 9



TRAFFIC VOLUMES
2024 Background Plus Site Trips Conditions
Evening Peak Hour

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OPERATIONAL ANALYSIS

The operational analysis for future traffic conditions was again conducted using Synchro analysis software, with outputs based on the analysis methodologies contained in the *HIGHWAY CAPACITY MANUAL, 6th Edition*. The analysis was prepared for the intersections' morning and evening peak hours.

The results of the operational analysis are summarized in Table 4 below. Detailed analysis worksheets are also included in the technical appendix.

Table 4 - Operational Analysis Summary: Year 2024 Future Conditions

Intersection	AM Peak Hour			PM Peak Hour		
	Delay	LOS	v/c*	Delay	LOS	v/c*
SE Powell Blvd. at SE 182nd Ave.						
2024 Background	41.3	D	0.90	54.6	D	0.86
2024 Background plus Site	41.6	D	0.91	56.5	E	0.87
SE 190th Dr. at SE Butler Rd.						
2024 Background*	8.8	A	0.49	7.3	A	0.54
2024 Background plus Site*	9.1	A	0.52	7.5	A	0.55
SE 190th Dr. at SE Richey Rd.						
2024 Background	21.4	C	0.75	59.6	F	0.98
2024 Background plus Site	26.3	D	0.81	108.4	F	1.12
2024 Background plus Site w/ SBRT	14.8	B	0.49	51.3	F	0.93
SE Foster Rd. at SE Richey Rd.						
2024 Background	26.3	D	0.76	47.8	E	0.89
2024 Background plus Site	28.7	D	0.79	63.8	F	0.97
2024 Background plus Site w/ WBLT	25.8	D	0.53	36.9	E	0.57
SE 190th Drive at North Site Access						
2024 Background plus Site	10.8	B	0.09	12.6	B	0.08
SE 190th Drive at South Site Access						
2024 Background plus Site	11.3	B	0.08	13.4	B	0.07

*Assumes completion of the traffic signal currently being installed.

It should be noted that although the intersection of SE Powell Boulevard at SE 182nd Avenue is shown as operating with average delays slightly above 55 seconds (into level of service E) during the evening peak hour, the intersection is expected to operate at level of service D. The intersection utilizes adaptive traffic control software which cannot be accurately modeled in out Synchro analysis. Assuming that the adaptive control software yields even a modest 3 percent reduction in delay, the resulting average intersection delays will be less than 55 seconds and the intersection will operate acceptably per City of Gresham standards. All individual turning movements are also projected to operate at level of service E or better and within capacity.



Based on analysis, the intersections of SE Powell Boulevard at SE 182nd Avenue, SE 190th Drive at SE Butler Road, and the two site access intersections on SE 190th Drive are projected to operate acceptably with no improvements required.

It should be noted that SE Richey Road is being converted from a collector roadway to a local street under the City of Gresham's Pleasant Valley TSP Refinement Plan. Future traffic volumes on this roadway would be projected to decrease upon completion of planned new east/west collector facilities that will be constructed as development within the area proceeds. As such, significant improvements such as construction of traffic signals should not be considered for implementation unless no other alternatives exist which would accommodate safe and efficient traffic flow in the interim. Accordingly, as consideration was given to appropriate operational mitigations for the study intersections on Richey Road, the goal was to meet the legal standard of no further degradation of these intersections.

The intersection of SE 190th Drive at SE Richey Road is projected to operate at level of service F and with volumes near intersection capacity during the evening peak hour under year 2024 background traffic conditions. With the addition of site trips from the proposed development, the intersection is projected to operate with volumes exceeding intersection capacity. If a southbound right-turn lane is provided at the intersection, operation is projected to improve as compared to background traffic conditions, with average delays on the highest-delay approach reduced to 51.5 seconds and all approaches operating at 93 percent of capacity or less. Accordingly, this mitigation is more than sufficient to offset the impacts of the proposed development and result in a net improvement to intersection operation as compared to background conditions.

The intersection of SE Foster Road at SE Richey Road is also projected to operate at level of service F and with volumes near intersection capacity during the evening peak hour under year 2024 background conditions. With the addition of site trips from the proposed development, the intersection is projected to operate with volumes exceeding intersection capacity. If a westbound left-turn lane is provided at the intersection, operation is again projected to improve. Notably, the combined average approach delay for the critical westbound approach is reduced from 63.8 seconds per vehicle to just 18.1 seconds per vehicle, with all intersection approaches operating within capacity. This proposed mitigation is more than sufficient to offset the impacts of the Veranda residential development and will result in a net improvement to intersection operation as compared to background conditions.

Based on the analysis, all study intersections will either operate acceptably per City of Gresham standards or operate with reduced delays and increased capacity as compared to background traffic conditions upon completion of the proposed development provided that a southbound right-turn lane is provided for the intersection of SE 190th Drive at SE Richey Road, and a westbound left-turn lane is provided for the intersection of SE Foster Road at SE Richey Road. No other mitigations are recommended in conjunction with the proposed development.

Based on a queuing simulation model, the projected 95th percentile queue lengths for the proposed new turn lanes were 105 feet for the southbound right-turn lane serving SE 190th Drive at SE Richey Road, and 161 feet for the westbound left-turn lane serving SE Foster Road at SE Richey Road. Accordingly, the turn lanes provided should accommodate these minimum storage requirements.



SAFETY ANALYSIS

CRASH DATA ANALYSIS

Using data obtained from the Oregon Department of Transportation, a review of the five most recent years of available crash history (from January 2015 through December 2019) was performed for the study intersections. The crash data was evaluated based on the number, type, and severity of collisions, as well as the intersection crash rate. Crash rates allow comparison of relative safety risks at intersections with different lane configurations, volumes, and traffic control devices by accounting for both the number of crashes that occur during the study period and the number of vehicles that traveled through the intersection during that period. Crash rates are calculated using the standard assumption that evening peak hour volumes are approximately 10 percent of the average daily traffic volume at an intersection. Crash rates in excess of one crash per million entering vehicle may be indicative of design deficiencies.

The intersection of SE Powell Boulevard at SE 182nd Avenue had 59 reported collisions during the five-year analysis period. These included 30 rear-end collisions, 14 turning-movement collisions, 7 angle collisions, 2 pedestrian collisions, 2 sideswipe-overtaking collisions, 1 backing collision, 1 sideswipe-meeting collision, 1 fixed-object collision, and 1 head-on collision. The crashes resulted in no fatalities; however, there was 1 incapacitating injury along with 5 non-incapacitating injuries and 52 reports of a “possible injury/complaint of pain”. The pedestrian collisions resulted in non-incapacitating injuries to the pedestrians. The incapacitating injury occurred when a westbound motorcycle traveling along SE Powell Boulevard was rear-ended by a westbound vehicle that was following too closely. The crash rate for the intersection was calculated to be 0.88 crashes per million entering vehicles.

The intersection of SW Pleasant View Drive/SE 190th Drive at SE Giese Road/SW Butler Road had 37 reported crashes during the five-year analysis period. These included 27 angle collisions, 5 rear-end collisions, 3 turning-movement collisions and 2 fixed-object collisions. The crashes resulted in no incapacitating injuries or fatalities. There were four non-incapacitating injuries reported, as well as 17 reports of a “possible injury/complaint of pain”. The crash rate for the intersection was calculated to be 1.34 crashes per million entering vehicles.

The intersection of SE 190th Drive at SE Richey Road had ten reported collisions during the five-year analysis period. These included seven turning-movement collisions, one angle collision, one rear-end collision, and one fixed-object collision. The crashes resulted in no fatalities or serious injuries; however, there were two non-incapacitating injuries and five reports of a “possible injury/complaint of pain”. The crash rate for the intersection was calculated to be 0.49 crashes per million entering vehicles.

The intersection of SE Foster Road at SE Richey Road had 17 reported collisions during the five-year analysis period. These included ten turning-movement collisions, six rear-end collisions and one fixed-object collision. The crashes resulted in no fatalities or serious injuries. There were four non-incapacitating injuries and 12 reports of a “possible injury/complaint of pain”. The crash rate for the intersection was calculated to be 0.68 crashes per million entering vehicles.



Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of SE 190th Drive at SW Butler Road had an unusually high crash rate primarily consisting of angle collisions. Completion of the traffic signal currently being installed is projected to reduce the incidence of angle collisions since drivers responding appropriately to the signal indications would not experience angle conflicts within the intersection.

INTERSECTION SIGHT DISTANCE ANALYSIS

Intersection sight distance was examined for each of the proposed new site access locations on SE 190th Drive.

Intersection sight distance is a design parameter which is intended to provide sufficient sight distance to allow drivers approaching an intersection on a stop-controlled minor-street approach to choose when to enter the major street without requiring through traffic on the major street to slow significantly. Intersection sight distance is measured from a position 15 feet behind the edge of the traveled way a driver's eye height of 3.5 feet above the roadway to a position 3.5 feet above the surface of the oncoming travel lanes.

In the Pleasant Valley TSP Refinement, SE 190th Drive is planned to have a Major Arterial, with a design speed of 45 mph. Based on this design speed, a minimum of 500 feet of intersection sight distance should be provided in each direction.

From the north site access (857 feet south of the centerline of Richey Road), intersection sight distance was measured to be greater than 800 feet in each direction. Accordingly, adequate sight distance is available in both directions for this proposed site access intersection.

Initially, the south site access was proposed to be located approximately 1,300 feet south of the centerline of Richey Road. However, from this position sight distance was restricted to just 430 feet to the south by a crest vertical curve. Since sight lines to the south were restricted to less than the desired minimum, the south side access was re-aligned to a position where adequate sight lines could be attained in each direction. From a position 1,173 feet south of the centerline of Richey Road, intersection sight distances of at least 500 feet are attainable in both directions.

Based on the sight distance analysis and the revised location for the south site access, adequate sight distance is now projected to be available for safe and efficient operation at both of the proposed site access intersections.



CONCLUSIONS

Based on the operational analysis, all study intersections will either operate acceptably per City of Gresham standards or operate with reduced delays and increased capacity as compared to background conditions upon completion of the proposed development provided that a southbound right-turn lane is provided for the intersection of SE 190th Drive at SE Richey Road, and a westbound left-turn lane is provided for the intersection of SE Foster Road at SE Richey Road. No other mitigations are recommended in conjunction with the proposed development.

Based on a queuing simulation model, the projected 95th percentile queue lengths for the proposed new turn lanes were 105 feet for the southbound right-turn lane serving SE 190th Drive at SE Richey Road, and 161 feet for the westbound left-turn lane serving SE Foster Road at SE Richey Road. Accordingly, the turn lanes provided should accommodate these minimum storage requirements.

Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of SE 190th Drive at SW Butler Road has a calculated crash rate of 1.34 crashes per million entering vehicle, consisting primarily of angle collisions. However, the traffic signal currently being installed is expected to significantly reduce the frequency of these crashes. Based on the crash data analysis, no other safety mitigations are recommended in conjunction with the proposed development.

Intersection sight distance was evaluated for the two new site access intersections on SE 190th Drive. The proposed site access locations are 857 feet and 1,173 feet south of the centerline of Richey Road. From these locations, adequate sight distance is projected to be available for safe and efficient operation of the intersections.

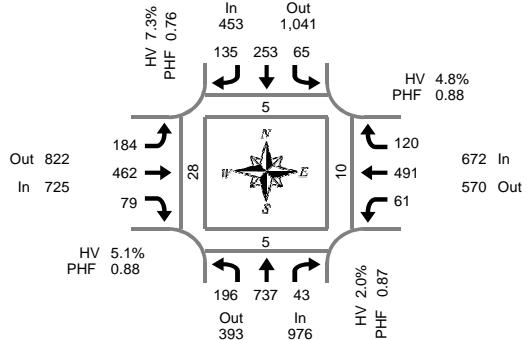


APPENDIX

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE 182nd Ave & SE Powell Blvd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:00 AM to 8:00 AM

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	15	47	6	0	7	22	9	0	11	30	2	0	2	36	11	0	198
7:05 AM	10	51	2	0	4	12	10	0	10	25	8	0	7	56	7	0	202
7:10 AM	27	66	1	0	3	21	16	0	22	35	7	0	3	44	10	0	255
7:15 AM	11	72	4	0	0	12	5	0	10	44	10	0	4	50	10	0	232
7:20 AM	19	76	3	0	8	27	10	0	19	34	10	0	2	33	8	0	249
7:25 AM	13	65	1	0	6	21	9	0	25	41	12	0	10	38	5	0	246
7:30 AM	17	69	3	0	8	31	18	1	14	32	6	0	4	47	17	0	266
7:35 AM	15	68	6	1	3	19	19	0	22	43	4	0	6	35	12	0	252
7:40 AM	21	73	4	0	8	30	13	0	10	43	2	0	1	35	7	0	247
7:45 AM	13	53	2	0	4	16	7	0	11	55	6	0	8	48	11	0	234
7:50 AM	19	56	3	0	3	20	9	0	17	42	7	0	5	25	10	0	216
7:55 AM	16	41	8	0	11	22	10	0	13	38	5	0	9	44	12	0	229
8:00 AM	18	43	5	0	5	18	9	0	18	35	4	0	9	20	9	0	193
8:05 AM	14	41	5	1	7	19	12	0	18	29	5	0	1	36	14	0	201
8:10 AM	10	43	4	0	16	18	9	0	6	33	7	0	10	37	9	0	202
8:15 AM	20	38	4	0	7	16	10	0	11	43	6	0	3	41	11	0	210
8:20 AM	17	35	1	0	7	23	9	0	21	37	4	0	3	42	7	0	206
8:25 AM	15	45	3	0	16	11	6	0	4	45	13	0	4	39	7	0	208
8:30 AM	14	40	9	0	10	24	13	0	14	50	12	0	6	42	11	1	245
8:35 AM	17	53	4	0	12	19	4	0	10	42	13	0	9	25	9	0	217
8:40 AM	7	36	2	0	4	19	3	0	10	43	11	0	9	55	12	0	211
8:45 AM	14	43	11	0	4	18	15	0	10	40	2	0	4	39	7	0	207
8:50 AM	10	38	11	0	15	22	8	0	15	31	4	0	5	35	4	0	198
8:55 AM	16	26	0	0	6	34	13	0	11	37	6	0	5	42	5	0	201
Total Survey	368	1,218	102	2	174	494	246	1	332	927	166	0	129	944	225	1	5,325

Pedestrians Crosswalk			
North	South	East	West
0	0	0	2
0	1	1	3
0	1	0	2
0	0	0	0
0	0	0	6
0	0	4	1
0	0	0	4
2	3	4	3
0	0	0	4
0	0	0	4
0	0	1	1
1	0	0	0
0	0	1	1
0	0	1	2
1	0	1	1
0	0	0	1
0	0	0	1
1	0	0	2

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	52	164	9	0	14	55	35	0	43	90	17	0	12	136	28	0	655
7:15 AM	43	213	8	0	14	60	24	0	54	119	32	0	16	121	23	0	727
7:30 AM	53	210	13	1	19	80	50	1	46	118	12	0	11	117	36	0	765
7:45 AM	48	150	13	0	18	58	26	0	41	135	18	0	22	117	33	0	679
8:00 AM	42	127	14	1	28	55	30	0	42	97	16	0	20	93	32	0	596
8:15 AM	52	118	8	0	30	50	25	0	36	125	23	0	10	122	25	0	624
8:30 AM	38	129	15	0	26	62	20	0	34	135	36	0	24	122	32	1	673
8:45 AM	40	107	22	0	25	74	36	0	36	108	12	0	14	116	16	0	606
Total Survey	368	1,218	102	2	174	494	246	1	332	927	166	0	129	944	225	1	5,325

Pedestrians Crosswalk			
North	South	East	West
0	2	1	7
0	0	4	7
2	3	4	11
3	0	1	3
2	0	1	1
2	0	4	6
2	0	1	3
1	0	0	4

Peak Hour Summary

7:00 AM to 8:00 AM

By Approach	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	976	393	1,369	1	453	1,041	1,494	1	725	822	1,547	0	672	570	1,242	0	2,826
%HV	2.0%				7.3%				5.1%				4.8%				4.3%
PHF	0.87				0.76				0.88				0.88				0.92

Pedestrians Crosswalk			
North	South	East	West
5	5	10	28
7	3	10	22
9	3	10	21
9	0	7	13
7	0	6	14

Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	196	737	43	1	65	253	135	1	184	462	79	0	61	491	120	0	2,826
7:15 AM	186	700	48	2	79	253	130	1	183	469	78	0	69	448	124	0	2,767
7:30 AM	195	605	48	2	95	243	131	1	165	475	69	0	63	449	126	0	2,664
7:45 AM	180	524	50	1	102	225	101	0	153	492	93	0	76	454	122	1	2,572
8:00 AM	172	481	59	1	109	241	111	0	148	465	87	0	68	453	105	1	2,499

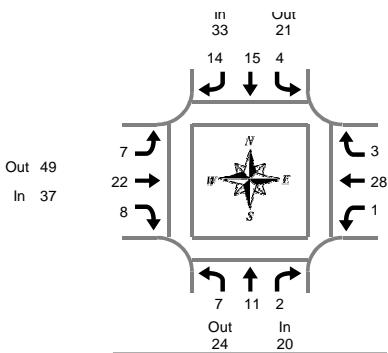
Pedestrians Crosswalk			

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



SE 182nd Ave & SE Powell Blvd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:00 AM to 8:00 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	2	0	1	3	1	2	3	6	1	2	0	3	0	1	0	1	13
7:05 AM	0	0	0	0	1	2	5	8	1	3	1	5	0	2	0	2	15
7:10 AM	0	0	0	0	0	2	1	3	0	4	1	5	0	0	0	0	8
7:15 AM	0	0	0	0	0	2	0	2	0	1	3	4	0	1	0	1	7
7:20 AM	0	1	0	1	0	1	0	1	1	3	0	4	0	4	0	4	10
7:25 AM	0	3	0	3	0	1	0	1	1	1	1	3	1	0	0	1	8
7:30 AM	3	1	0	4	0	0	2	2	1	0	0	1	0	6	0	6	13
7:35 AM	1	1	0	2	0	0	1	1	1	2	0	3	0	1	1	2	8
7:40 AM	0	2	0	2	1	1	1	3	1	1	0	2	0	4	1	5	12
7:45 AM	0	1	1	2	0	0	0	0	1	1	2	0	1	0	1	0	5
7:50 AM	0	1	0	1	1	4	0	5	0	4	1	5	0	2	1	3	14
7:55 AM	1	1	0	2	0	0	1	1	0	0	0	0	0	6	0	6	9
8:00 AM	1	2	0	3	0	0	1	1	1	2	0	3	0	1	0	1	8
8:05 AM	1	0	1	2	0	2	2	4	1	0	0	1	0	2	0	2	9
8:10 AM	0	0	0	0	2	2	0	4	0	2	1	3	0	1	0	1	8
8:15 AM	0	0	0	0	0	3	0	3	0	4	0	4	0	5	3	8	15
8:20 AM	0	1	0	1	1	0	0	1	1	1	0	2	0	5	0	5	9
8:25 AM	0	1	1	2	1	1	1	3	0	4	1	5	0	2	1	3	13
8:30 AM	1	5	0	6	0	0	1	1	0	1	0	1	0	2	0	2	10
8:35 AM	1	1	0	2	1	1	0	2	0	1	0	1	0	1	2	3	8
8:40 AM	0	0	0	0	0	0	0	0	0	4	0	4	0	1	0	1	5
8:45 AM	0	2	0	2	0	0	2	2	1	5	0	6	0	5	0	5	15
8:50 AM	0	0	1	1	2	1	0	3	0	1	0	1	0	1	0	1	6
8:55 AM	1	0	0	1	0	2	0	2	0	2	0	2	0	4	0	4	9
Total Survey	12	23	5	40	11	27	21	59	11	49	10	70	1	58	9	68	237

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	2	0	1	3	2	6	9	17	2	9	2	13	0	3	0	3	36
7:15 AM	0	4	0	4	0	4	0	4	2	5	4	11	1	5	0	6	25
7:30 AM	4	4	0	8	1	1	4	6	3	3	0	6	0	11	2	13	33
7:45 AM	1	3	1	5	1	4	1	6	0	5	2	7	0	9	1	10	28
8:00 AM	2	2	1	5	2	4	3	9	2	4	1	7	0	4	0	4	25
8:15 AM	0	2	1	3	2	4	1	7	1	9	1	11	0	12	4	16	37
8:30 AM	2	6	0	8	1	1	1	3	0	6	0	6	0	4	2	6	23
8:45 AM	1	2	1	4	2	3	2	7	1	8	0	9	0	10	0	10	30
Total Survey	12	23	5	40	11	27	21	59	11	49	10	70	1	58	9	68	237

Heavy Vehicle Peak Hour Summary

7:00 AM to 8:00 AM

By Approach	Northbound SE 182nd Ave			Southbound SE 182nd Ave			Eastbound SE Powell Blvd			Westbound SE Powell Blvd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	20	24	44	33	21	54	37	49	86	32	28	60	122
PHF	0.56		0.49			0.66			0.62			0.85	

By Movement	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	7	11	2	20	4	15	14	33	7	22	8	37	1	28	3	32	122
PHF	0.44	0.55	0.50	0.56	0.50	0.63	0.39	0.49	0.58	0.61	0.40	0.66	0.25	0.64	0.38	0.62	0.85

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	7	11	2	20	4	15	14	33	7	22	8	37	1	28	3	32	122
7:15 AM	7	13	2	22	4	13	8	25	7	17	7	31	1	29	3	33	111
7:30 AM	7	11	3	21	6	13	9	28	6	21	4	31	0	36	7	43	123
7:45 AM	5	13	3	21	6	13	6	25	3	24	4	31	0	29	7	36	113
8:00 AM	5	12	3	20	7	12	7	26	4	27	2	33	0	30	6	36	115

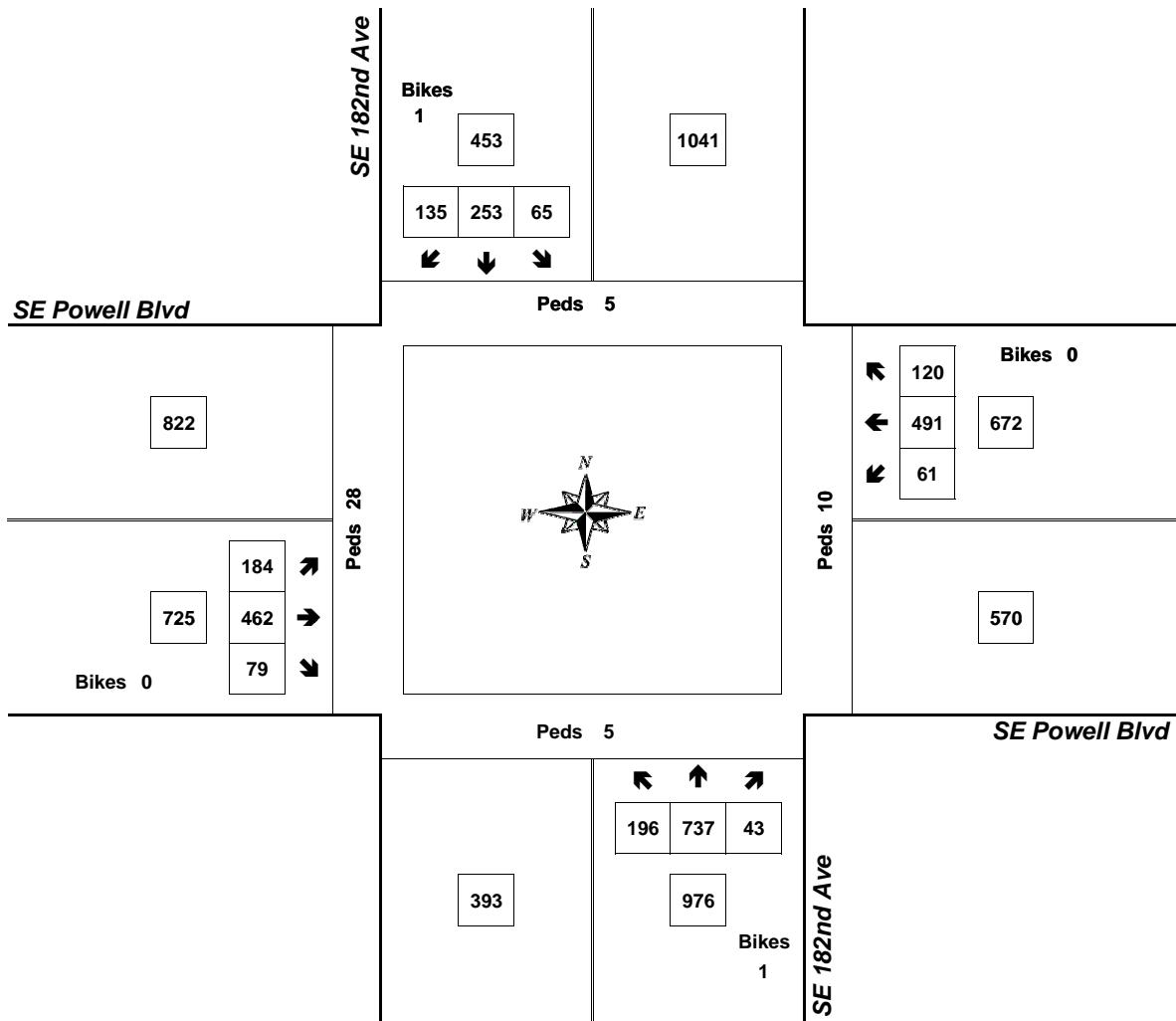
Peak Hour Summary



Clay Carney
(503) 833-2740

SE 182nd Ave & SE Powell Blvd

7:00 AM to 8:00 AM
Tuesday, May 07, 2019



Count Period: 7:00 AM to 9:00 AM

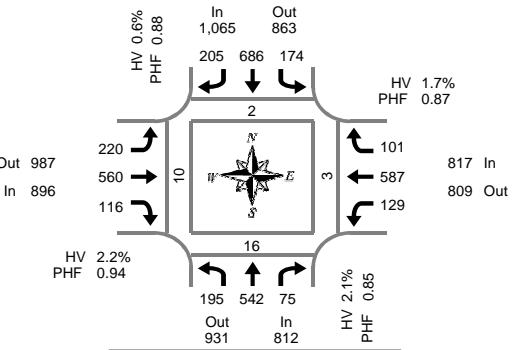
Total Vehicle Summary



Clay Carney
(503) 833-2740

SE 182nd Ave & SE Powell Blvd

Tuesday, May 07, 2019
4:00 PM to 6:00 PM



Peak Hour Summary 4:35 PM to 5:35 PM

5-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	20	35	2	0	18	46	11	0	17	46	16	0	9	36	8	0	264
4:05 PM	12	38	6	0	14	75	14	0	13	51	15	0	17	54	5	0	314
4:10 PM	21	50	3	0	25	58	19	0	14	46	12	0	9	45	9	0	311
4:15 PM	10	32	5	0	14	71	16	0	10	56	10	0	9	51	12	0	296
4:20 PM	21	30	3	0	15	38	14	0	21	50	12	0	14	50	8	0	276
4:25 PM	11	56	6	0	10	73	18	0	16	35	5	0	14	18	6	0	268
4:30 PM	19	47	1	0	18	56	10	0	11	42	10	0	2	36	10	0	262
4:35 PM	11	38	10	0	17	56	12	0	27	57	11	0	21	47	10	0	317
4:40 PM	15	33	3	0	23	48	13	1	21	54	8	0	3	49	15	0	285
4:45 PM	15	66	8	0	10	60	17	0	20	34	7	0	18	31	7	0	293
4:50 PM	14	34	3	0	16	53	25	0	26	58	11	0	8	46	14	0	308
4:55 PM	21	74	4	0	15	65	15	0	13	42	6	0	8	31	9	0	303
5:00 PM	12	42	7	0	15	59	20	0	21	47	5	1	13	54	8	1	303
5:05 PM	29	38	4	0	12	49	18	0	15	49	16	0	5	65	6	0	306
5:10 PM	13	41	5	0	15	90	26	0	21	30	11	0	15	49	7	0	323
5:15 PM	13	31	5	0	6	49	18	1	12	63	10	0	12	68	7	0	294
5:20 PM	16	64	10	0	18	57	15	0	12	38	8	0	7	46	4	0	295
5:25 PM	16	38	9	0	15	48	11	0	17	54	13	0	11	47	10	0	289
5:30 PM	20	43	7	0	12	52	15	0	15	34	10	0	8	54	4	0	274
5:35 PM	13	43	3	0	11	71	22	0	23	37	6	0	20	38	4	0	291
5:40 PM	12	27	2	0	15	56	13	0	14	66	10	1	9	49	6	0	279
5:45 PM	15	55	4	0	23	66	13	0	14	40	8	0	11	24	8	0	281
5:50 PM	13	50	5	0	8	32	9	0	24	48	8	0	15	45	9	0	266
5:55 PM	16	36	3	0	21	49	21	0	14	60	12	0	7	47	11	0	297
Total Survey	378	1,041	118	0	366	1,377	385	2	411	1,137	240	2	265	1,080	197	1	6,995

Pedestrians Crosswalk			
North	South	East	West
0	0	2	1
0	3	1	1
0	0	0	0
1	0	0	0
0	1	0	1
0	0	0	0
1	0	0	1
0	2	1	2
0	0	0	0
0	1	0	0
0	0	0	0
0	3	0	1
0	0	0	0
0	0	0	0
0	6	1	3
0	0	0	0
2	0	0	1
2	0	3	1
2	2	1	3
0	0	0	1
0	3	2	3
10	25	12	23

15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	53	123	11	0	57	179	44	0	44	143	43	0	35	135	22	0	889
4:15 PM	42	118	14	0	39	182	48	0	47	141	27	0	37	119	26	0	840
4:30 PM	45	118	14	0	58	160	35	1	59	153	29	0	26	132	35	0	864
4:45 PM	50	174	15	0	41	178	57	0	59	134	24	0	34	108	30	0	904
5:00 PM	54	121	16	0	42	198	64	0	57	126	32	1	33	168	21	1	932
5:15 PM	45	133	24	0	39	154	44	1	41	155	31	0	30	161	21	0	878
5:30 PM	45	113	12	0	38	179	50	0	52	137	26	1	37	141	14	0	844
5:45 PM	44	141	12	0	52	147	43	0	52	148	28	0	33	116	28	0	844
Total Survey	378	1,041	118	0	366	1,377	385	2	411	1,137	240	2	265	1,080	197	1	6,995

Pedestrians Crosswalk			
North	South	East	West
0	3	3	2
1	1	0	1
1	2	1	3
1	3	0	3
1	4	1	2
0	7	1	3
4	0	3	2
2	5	3	7
10	25	12	23

Peak Hour Summary 4:35 PM to 5:35 PM

By Approach	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	812	931	1,743	0	1,065	863	1,928	2	896	987	1,883	1	817	809	1,626	1	3,590
%HV	2.1%				0.6%				2.2%				1.7%				1.6%
PHF	0.85				0.88				0.94				0.87				0.96

By Movement	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	195	542	75	812	174	686	205	1,065	220	560	116	896	129	587	101	817	3,590
%HV	2.6%	1.8%	2.7%	2.1%	1.1%	0.1%	1.5%	0.6%	1.8%	2.9%	0.0%	2.2%	0.0%	1.9%	3.0%	1.7%	1.6%
PHF	0.79	0.78	0.72	0.85	0.87	0.87	0.80	0.88	0.81	0.90	0.78	0.94	0.77	0.81	0.70	0.87	0.96

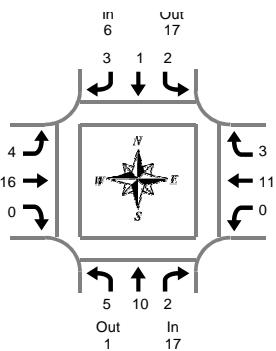
Pedestrians Crosswalk			
North	South	East	West
3	9	4	9
4	10	2	9
3	16	3	11
6	14	5	10
7	16	8	14

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



SE 182nd Ave & SE Powell Blvd

Tuesday, May 07, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:35 PM to 5:35 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	2	1	0	3	0	0	0	0	0	0	0	0	0	1	0	1	4
4:05 PM	1	2	1	4	2	2	1	5	0	2	1	3	0	1	0	1	13
4:10 PM	0	1	0	1	1	0	0	1	0	3	0	3	0	3	0	3	8
4:15 PM	1	2	0	3	0	3	0	3	0	1	0	1	0	1	0	1	8
4:20 PM	0	1	0	1	1	1	0	2	1	3	1	5	0	1	1	2	10
4:25 PM	1	1	0	2	0	0	0	0	0	0	0	1	0	0	0	0	3
4:30 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	1	0	1	2
4:35 PM	0	0	1	1	0	1	0	1	0	3	0	3	0	2	1	3	8
4:40 PM	1	1	0	2	0	0	1	1	1	0	2	0	1	0	1	1	6
4:45 PM	0	3	0	3	0	0	0	0	0	0	0	1	0	0	0	0	4
4:50 PM	0	0	0	0	0	0	0	0	0	1	0	2	0	0	0	0	2
4:55 PM	0	1	0	1	0	0	0	0	0	1	0	1	0	1	1	2	4
5:00 PM	1	1	0	2	0	0	0	0	0	1	2	0	3	0	2	0	7
5:05 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1
5:10 PM	0	1	0	1	1	0	0	1	0	2	0	2	0	2	0	2	6
5:15 PM	1	2	0	3	0	0	1	1	0	2	0	2	0	1	0	1	7
5:20 PM	0	1	1	2	1	0	0	1	0	1	0	1	0	0	1	1	5
5:25 PM	1	0	0	1	0	0	0	0	0	2	0	2	0	0	0	0	3
5:30 PM	1	0	0	1	0	0	0	0	0	1	0	1	0	2	0	2	4
5:35 PM	1	2	0	3	0	0	0	0	0	1	0	1	2	0	1	0	6
5:40 PM	0	1	0	1	1	1	0	2	1	1	0	2	0	2	0	2	7
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3
5:50 PM	1	1	0	2	0	0	0	0	0	1	0	0	1	0	4	0	7
5:55 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Survey	12	23	3	38	7	8	4	19	9	27	3	39	0	29	4	33	129

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	3	4	1	8	3	2	1	6	0	5	1	6	0	5	0	5	25
4:15 PM	2	4	0	6	1	4	0	5	2	4	1	7	0	2	1	3	21
4:30 PM	1	2	1	4	0	1	1	2	1	4	0	5	0	4	1	5	16
4:45 PM	0	4	0	4	0	0	0	0	2	2	0	4	0	1	1	2	10
5:00 PM	1	2	0	3	1	0	1	2	1	4	0	5	0	4	0	4	14
5:15 PM	2	3	1	6	1	0	1	2	0	5	0	5	0	1	1	2	15
5:30 PM	2	3	0	5	1	1	0	2	2	2	1	5	0	5	0	5	17
5:45 PM	1	1	0	2	0	0	0	0	1	1	0	2	0	7	0	7	11
Total Survey	12	23	3	38	7	8	4	19	9	27	3	39	0	29	4	33	129

Heavy Vehicle Peak Hour Summary

4:35 PM to 5:35 PM

By Approach	Northbound SE 182nd Ave			Southbound SE 182nd Ave			Eastbound SE Powell Blvd			Westbound SE Powell Blvd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	17	1	18	6	17	23	20	19	39	14	20	34	57
PHF	0.71		0.50			0.83			0.88			0.79	

By Movement	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	5	10	2	17	2	1	3	6	4	16	0	20	0	11	3	14	57
PHF	0.63	0.63	0.50	0.71	0.25	0.25	0.38	0.50	0.33	0.80	0.00	0.83	0.00	0.69	0.75	0.88	0.79

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 182nd Ave				Southbound SE 182nd Ave				Eastbound SE Powell Blvd				Westbound SE Powell Blvd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	6	14	2	22	4	7	2	13	5	15	2	22	0	12	3	15	72
4:15 PM	4	12	1	17	2	5	2	9	6	14	1	21	0	11	3	14	61
4:30 PM	4	11	2	17	2	1	3	6	4	15	0	19	0	10	3	13	55
4:45 PM	5	12	1	18	3	1	2	6	5	13	1	19	0	11	2	13	56
5:00 PM	6	9	1	16	3	1	2	6	4	12	1	17	0	17	1	18	57

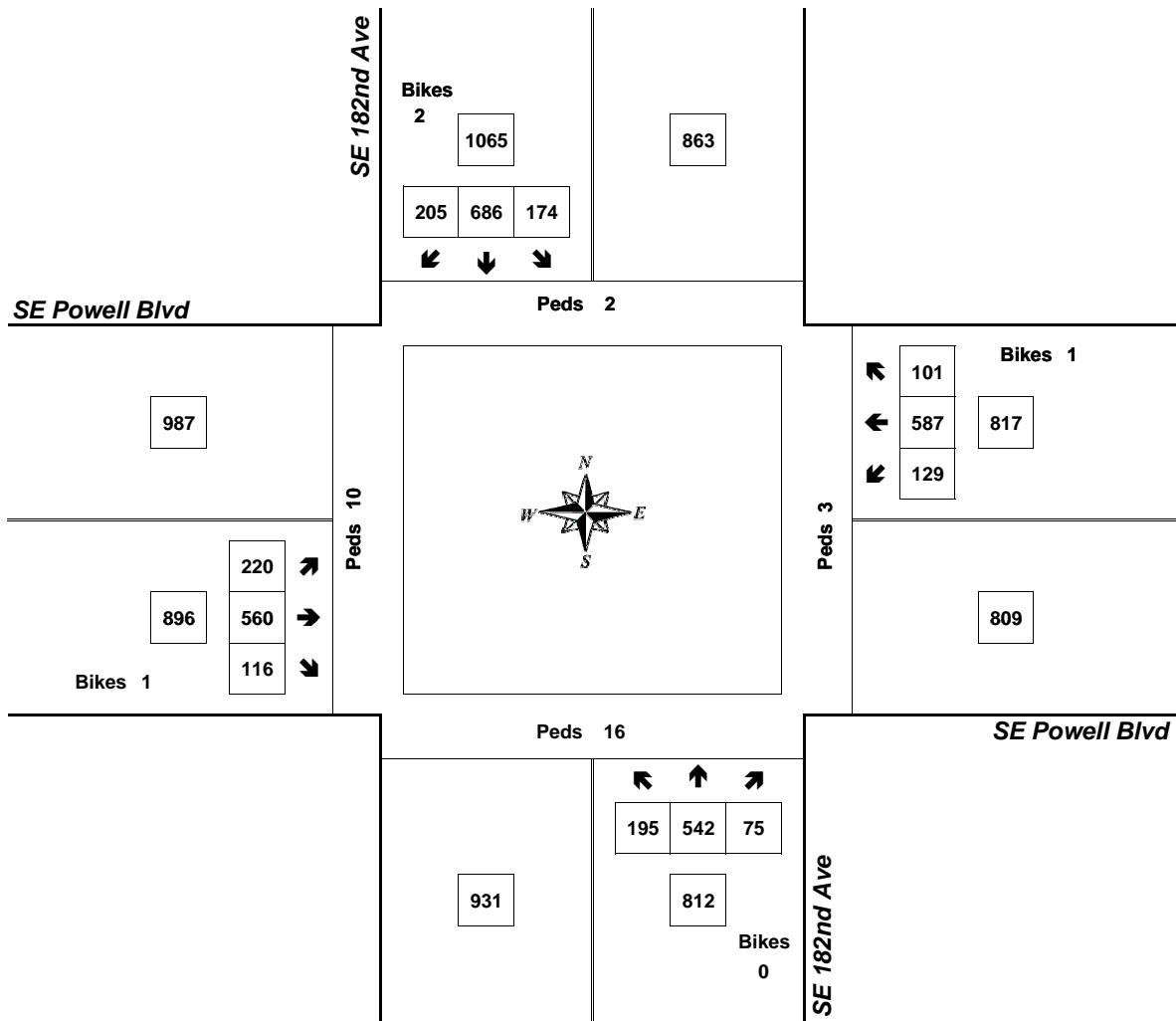
Peak Hour Summary



Clay Carney
(503) 833-2740

SE 182nd Ave & SE Powell Blvd

4:35 PM to 5:35 PM
Tuesday, May 07, 2019



Approach	PHF	HV%	Volume
EB	0.94	2.2%	896
WB	0.87	1.7%	817
NB	0.85	2.1%	812
SB	0.88	0.6%	1,065
Intersection	0.96	1.6%	3,590

Count Period: 4:00 PM to 6:00 PM

Total Vehicle Summary



Clay Carney
(503) 833-2740

Clay Carney

Clay Caney
(503) 833-2740

Clay Carney
(503) 833-3740

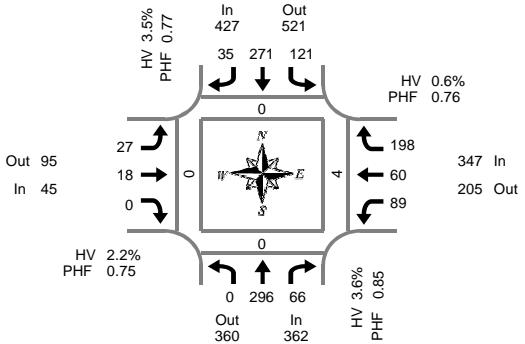
Clay Carney
(703) 555-0718

(503) 833-2740

SW Pleasant View Dr & SW Butler Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM



Peak Hour Summary
7:05 AM to 8:05 AM

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	0	12	3	0	6	24	1	0	0	1	0	0	11	3	8	0	69
7:05 AM	0	15	7	0	5	23	2	0	1	1	0	0	11	6	12	0	83
7:10 AM	0	29	10	0	6	16	3	0	0	1	0	0	10	0	13	0	88
7:15 AM	0	23	3	0	11	22	2	0	6	1	0	0	5	7	14	0	94
7:20 AM	0	26	7	0	11	24	5	0	2	2	0	0	4	4	18	0	103
7:25 AM	0	21	4	0	20	24	7	0	3	1	0	0	7	5	25	0	117
7:30 AM	0	18	5	0	13	32	2	0	0	2	0	0	9	4	24	0	109
7:35 AM	0	37	0	0	14	21	3	0	3	3	0	0	9	9	22	0	121
7:40 AM	0	37	2	0	15	23	1	0	4	1	0	0	5	6	23	0	117
7:45 AM	0	24	6	0	9	25	2	0	3	1	0	0	9	3	22	0	104
7:50 AM	0	19	5	0	6	20	4	0	2	0	0	0	8	8	16	0	88
7:55 AM	0	29	9	0	2	20	2	0	3	1	0	0	7	3	3	0	79
8:00 AM	0	18	8	0	9	21	2	0	0	4	0	0	5	5	6	0	78
8:05 AM	0	18	6	0	5	23	0	0	2	3	0	0	4	3	11	0	75
8:10 AM	0	19	3	0	4	17	4	0	1	0	0	0	3	1	9	0	61
8:15 AM	0	26	8	0	1	18	3	0	1	3	0	0	2	4	7	0	73
8:20 AM	0	26	3	0	2	18	4	0	2	2	0	0	8	8	8	0	81
8:25 AM	0	16	4	0	3	12	1	0	2	2	0	0	4	2	5	0	51
8:30 AM	0	22	9	0	1	15	2	0	5	2	0	0	4	2	10	0	72
8:35 AM	0	20	4	0	7	19	1	0	2	4	0	0	6	2	8	0	73
8:40 AM	0	29	5	0	5	13	4	0	1	4	0	0	3	1	7	0	72
8:45 AM	0	20	5	0	4	21	6	0	0	0	0	0	6	1	5	0	68
8:50 AM	1	22	2	0	5	13	1	1	4	3	0	0	7	5	6	0	69
8:55 AM	0	14	4	0	3	10	3	0	1	1	0	0	10	0	6	0	52
Total Survey	1	540	122	0	167	474	65	1	48	43	0	0	157	92	288	0	1,997

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	0	56	20	0	17	63	6	0	1	3	0	0	32	9	33	0	240
7:15 AM	0	70	14	0	42	70	14	0	11	4	0	0	16	16	57	0	314
7:30 AM	0	92	7	0	42	76	6	0	7	6	0	0	23	19	69	0	347
7:45 AM	0	72	20	0	17	65	8	0	8	2	0	0	24	14	41	0	271
8:00 AM	0	55	17	0	18	61	6	0	3	7	0	0	12	9	26	0	214
8:15 AM	0	68	15	0	6	48	8	0	5	7	0	0	14	14	20	0	205
8:30 AM	0	71	18	0	13	47	7	0	8	10	0	0	13	5	25	0	217
8:45 AM	1	56	11	0	12	44	10	1	5	4	0	0	23	6	17	0	189
Total Survey	1	540	122	0	167	474	65	1	48	43	0	0	157	92	288	0	1,997

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	4	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	1	0
0	0	0	0
0	0	5	0

Peak Hour Summary

**Peak Hour Summary
7:05 AM to 8:05 AM**

By Approach	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	362	360	722	0	427	521	948	0	45	95	140	0	347	205	552	0	1,181
%HV	3.6%				3.5%				2.2%				0.6%				2.6%
PHF	0.85				0.77				0.75				0.76				0.85

Pedestrians

Crosswalk

By Movement	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	0	296	66	362	121	271	35	427	27	18	0	45	89	60	198	347	1,181
%HV	0.0%	3.7%	3.0%	3.6%	5.0%	3.0%	2.9%	3.5%	3.7%	0.0%	0.0%	2.2%	0.0%	1.7%	0.5%	0.6%	2.6%
PHF	0.00	0.76	0.75	0.85	0.64	0.85	0.63	0.77	0.61	0.75	0.00	0.75	0.86	0.79	0.70	0.76	0.85

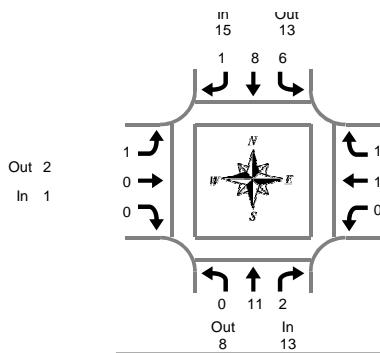
Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound				Southbound				Eastbound				Westbound				Interval Total
	SW Pleasant View Dr				SW Pleasant View Dr				SW Butler Rd				SW Butler Rd				
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	0	290	61	0	118	274	34	0	27	15	0	0	95	58	200	0	1,172
7:15 AM	0	289	58	0	119	272	34	0	29	19	0	0	75	58	193	0	1,146
7:30 AM	0	287	59	0	83	250	28	0	23	22	0	0	73	56	156	0	1,037
7:45 AM	0	266	70	0	54	221	29	0	24	26	0	0	63	42	112	0	907
8:00 AM	1	250	61	0	49	200	31	1	21	28	0	0	62	34	88	0	825

Pedestrians Crosswalk			
North	South	East	West
0	0	4	0
0	0	4	0
0	0	0	0
0	0	1	0
0	0	1	0

Heavy Vehicle Summary



SW Pleasant View Dr & SW Butler Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:05 AM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
7:10 AM	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	2
7:15 AM	0	1	0	1	2	3	0	5	0	0	0	0	0	0	0	0	6
7:20 AM	0	2	0	2	1	0	0	1	0	0	0	0	0	0	0	0	3
7:25 AM	0	1	0	1	1	1	1	3	0	0	0	0	0	0	1	1	5
7:30 AM	0	0	1	1	1	1	0	2	0	0	0	0	0	0	0	0	3
7:35 AM	0	2	0	2	0	1	0	1	0	0	0	0	0	1	0	1	4
7:40 AM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
7:45 AM	0	0	0	0	1	1	0	2	0	0	0	0	0	0	0	0	2
7:50 AM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
7:55 AM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:05 AM	0	1	1	2	0	1	0	1	0	0	0	0	0	0	0	0	3
8:10 AM	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	2
8:15 AM	0	0	1	1	0	1	0	1	0	1	0	1	0	0	0	0	3
8:20 AM	0	0	1	1	0	1	0	1	0	0	0	0	0	0	0	0	2
8:25 AM	0	2	0	2	1	2	0	3	0	1	0	1	0	0	0	0	6
8:30 AM	0	0	0	0	0	0	0	0	0	1	2	0	3	0	0	0	3
8:35 AM	0	1	0	1	0	2	0	2	0	2	0	2	0	0	0	0	5
8:40 AM	0	1	0	1	0	1	0	1	0	0	0	0	0	1	1	1	3
8:45 AM	0	0	1	1	0	1	0	1	0	0	0	0	0	0	0	0	2
8:50 AM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
8:55 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
Total Survey	0	17	7	24	7	20	1	28	2	6	0	8	0	1	2	3	63

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	1	1	2	0	1	0	1	0	0	0	0	0	0	0	0	3
7:15 AM	0	4	0	4	4	4	1	9	0	0	0	0	0	0	1	1	14
7:30 AM	0	4	1	5	1	2	0	3	0	0	0	0	0	1	0	1	9
7:45 AM	0	2	0	2	1	1	0	2	1	0	0	1	0	0	0	0	5
8:00 AM	0	2	2	4	0	1	0	1	0	0	0	0	0	0	0	0	5
8:15 AM	0	2	2	4	1	4	0	5	0	2	0	2	0	0	0	0	11
8:30 AM	0	2	0	2	0	3	0	3	1	4	0	5	0	0	1	1	11
8:45 AM	0	0	1	1	0	4	0	4	0	0	0	0	0	0	0	0	5
Total Survey	0	17	7	24	7	20	1	28	2	6	0	8	0	1	2	3	63

Heavy Vehicle Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SW Pleasant View Dr			Southbound SW Pleasant View Dr			Eastbound SW Butler Rd			Westbound SW Butler Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	13	8	21	15	13	28	1	2	3	2	8	10	31
PHF	0.65			0.42			0.25			0.25			0.55

By Movement	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	0	11	2	13	6	8	1	15	1	0	0	1	0	1	1	2	31
PHF	0.00	0.69	0.50	0.65	0.38	0.50	0.25	0.42	0.25	0.00	0.00	0.25	0.00	0.25	0.25	0.55	

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	11	2	13	6	8	1	15	1	0	0	1	0	1	1	2	31
7:15 AM	0	12	3	15	6	8	1	15	1	0	0	1	0	1	1	2	33
7:30 AM	0	10	5	15	3	8	0	11	1	2	0	3	0	1	0	1	30
7:45 AM	0	8	4	12	2	9	0	11	2	6	0	8	0	0	1	1	32
8:00 AM	0	6	5	11	1	12	0	13	1	6	0	7	0	0	1	1	32

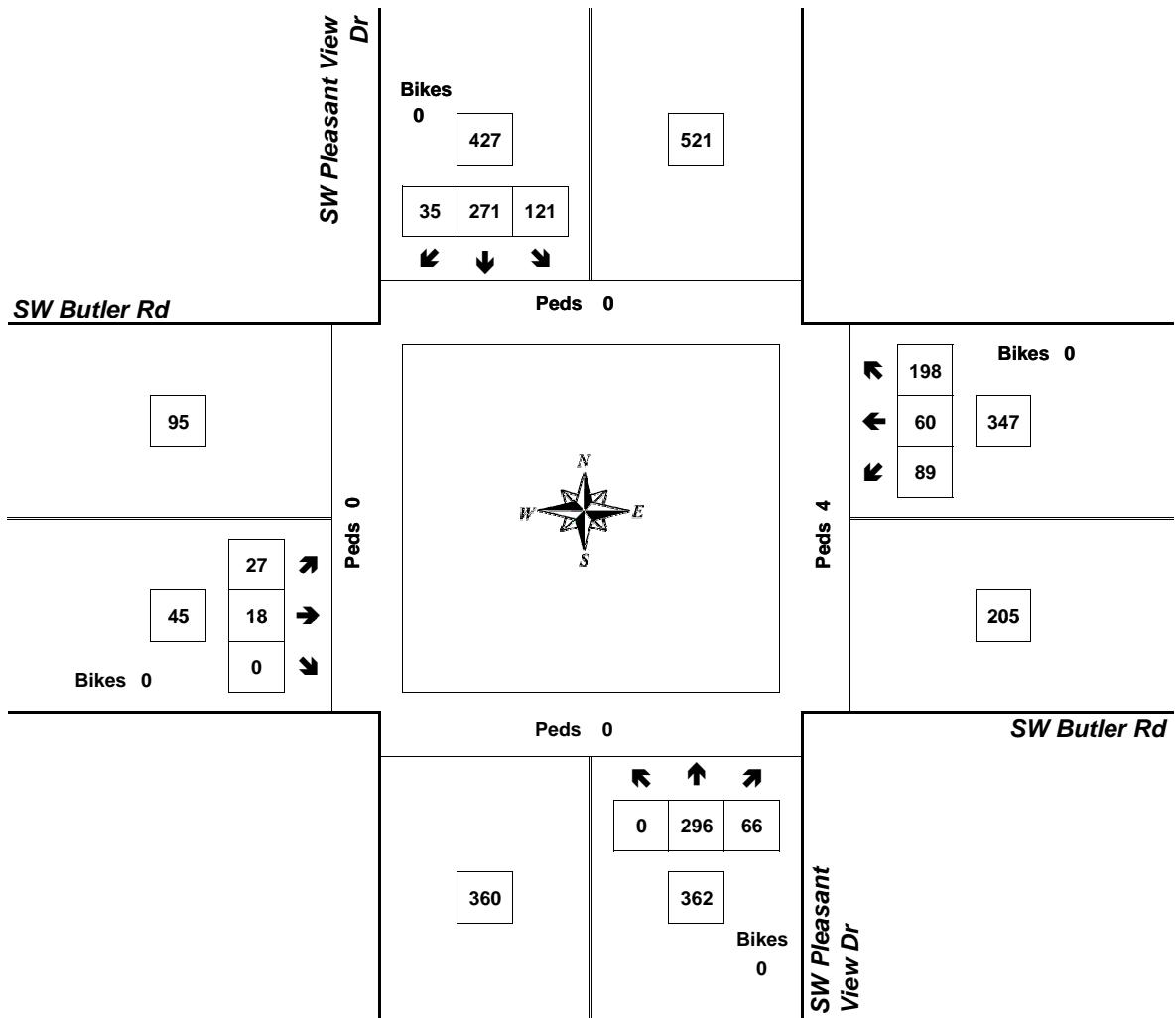
Peak Hour Summary



Clay Carney
(503) 833-2740

SW Pleasant View Dr & SW Butler Rd

7:05 AM to 8:05 AM
Tuesday, May 07, 2019



Approach	PHF	HV%	Volume
EB	0.75	2.2%	45
WB	0.76	0.6%	347
NB	0.85	3.6%	362
SB	0.77	3.5%	427
Intersection	0.85	2.6%	1,181

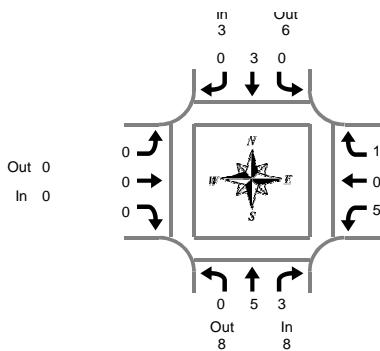
Count Period: 7:00 AM to 9:00 AM

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Canney
(503) 833-2740



SW Pleasant View Dr & SW Butler Rd

Tuesday, May 07, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:50 PM to 5:50 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	2	1	3	0	1	0	1	1	0	0	1	0	1	0	1	6
4:05 PM	0	2	1	3	0	1	1	2	0	0	0	0	0	0	1	1	6
4:10 PM	0	2	1	3	0	0	0	0	0	0	0	0	0	0	0	0	3
4:15 PM	0	1	0	1	0	0	0	0	0	0	0	0	2	0	0	2	3
4:20 PM	0	0	0	0	0	2	0	2	0	0	1	1	0	0	1	1	4
4:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
4:30 PM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
4:35 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
4:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	3	3
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	1	0	1	0	2
4:50 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
4:55 PM	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	2
5:00 PM	0	1	0	1	0	0	0	0	0	0	0	0	3	0	0	3	4
5:05 PM	0	1	1	2	0	0	0	0	0	0	0	0	0	0	0	0	2
5:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
5:20 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	1
5:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	1	0	1	0	1	0	1	0	0	0	0	1	0	0	1	3
5:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:40 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
5:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:55 PM	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	2
Total Survey	0	16	6	22	0	7	1	8	2	1	1	4	8	2	6	16	50

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	6	3	9	0	2	1	3	1	0	0	1	0	1	1	2	15
4:15 PM	0	1	0	1	0	2	0	2	0	0	1	1	3	0	1	4	8
4:30 PM	0	3	0	3	0	0	0	0	0	0	0	0	0	0	3	3	6
4:45 PM	0	2	1	3	0	0	0	0	0	1	0	0	1	0	1	0	5
5:00 PM	0	2	1	3	0	0	0	0	0	0	0	0	3	0	1	4	7
5:15 PM	0	0	1	1	0	0	0	0	0	0	0	0	1	0	0	1	2
5:30 PM	0	1	0	1	0	2	0	2	0	0	0	0	0	1	0	0	4
5:45 PM	0	1	0	1	0	1	0	1	0	1	0	1	0	0	0	0	3
Total Survey	0	16	6	22	0	7	1	8	2	1	1	4	8	2	6	16	50

Heavy Vehicle Peak Hour Summary

4:50 PM to 5:50 PM

By Approach	Northbound SW Pleasant View Dr			Southbound SW Pleasant View Dr			Eastbound SW Butler Rd			Westbound SW Butler Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	8	8	16	3	6	9	0	0	0	6	3	9	17
PHF	0.40		0.38	0.00	0.38	0.00	0.00	0.00	0.00	0.38		0.53	

By Movement	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	0	5	3	8	0	3	0	3	0	0	0	0	5	0	1	6	17
PHF	0.00	0.42	0.38	0.40	0.00	0.38	0.00	0.38	0.00	0.00	0.00	0.00	0.42	0.00	0.25	0.38	0.53

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SW Pleasant View Dr				Southbound SW Pleasant View Dr				Eastbound SW Butler Rd				Westbound SW Butler Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	12	4	16	0	4	1	5	2	0	1	3	3	2	5	10	34
4:15 PM	0	8	2	10	0	2	0	2	1	0	1	2	6	1	5	12	26
4:30 PM	0	7	3	10	0	0	0	0	1	0	0	1	4	1	4	9	20
4:45 PM	0	5	3	8	0	2	0	2	1	0	0	1	5	1	1	7	18
5:00 PM	0	4	2	6	0	3	0	3	0	1	0	1	5	0	1	6	16

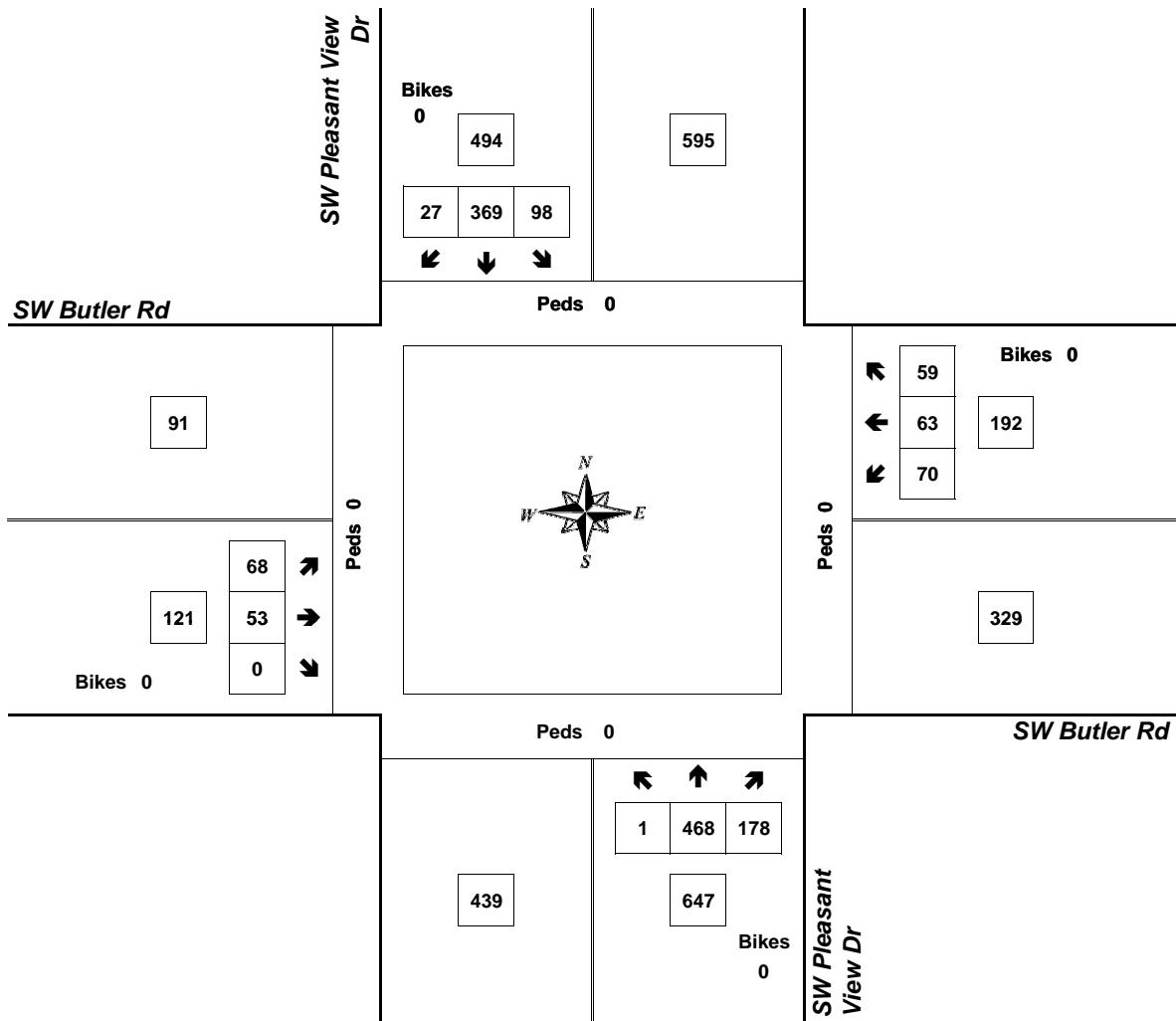
Peak Hour Summary



Clay Carney
(503) 833-2740

SW Pleasant View Dr & SW Butler Rd

4:50 PM to 5:50 PM
Tuesday, May 07, 2019

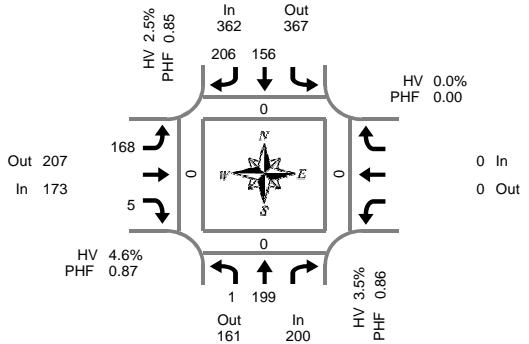


Count Period: 4:00 PM to 6:00 PM

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE 190th Ave & SE Richey Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Bikes	T	R	Bikes	L	R	Bikes			Bikes	
7:00 AM	0	7	0	9	18	0	7	0	0			0	41
7:05 AM	0	14	0	19	21	0	8	0	0			0	62
7:10 AM	0	24	0	10	17	0	15	1	0			0	67
7:15 AM	0	17	0	6	19	0	11	0	0			0	53
7:20 AM	0	17	0	12	15	0	12	1	0			0	57
7:25 AM	1	13	0	14	16	0	16	0	0			0	60
7:30 AM	0	15	0	12	28	0	11	0	0			0	66
7:35 AM	0	22	0	14	22	0	21	0	0			0	79
7:40 AM	0	16	0	15	10	0	14	1	0			0	56
7:45 AM	0	16	0	15	18	0	14	0	0			0	63
7:50 AM	0	13	0	16	13	0	14	1	0			0	57
7:55 AM	0	17	0	13	16	0	21	0	0			0	67
8:00 AM	0	15	0	10	11	0	11	1	0			0	48
8:05 AM	1	18	0	13	13	0	10	0	0			0	55
8:10 AM	0	13	0	9	14	0	3	0	0			0	39
8:15 AM	0	16	0	13	10	0	16	0	0			0	55
8:20 AM	0	22	0	9	15	0	9	0	0			0	55
8:25 AM	0	12	0	6	12	0	9	0	0			0	39
8:30 AM	0	19	0	11	8	0	16	0	0			0	54
8:35 AM	0	13	0	8	17	0	9	0	0			0	47
8:40 AM	0	10	0	7	9	0	26	0	0			0	52
8:45 AM	0	14	0	11	15	0	9	0	0			0	49
8:50 AM	0	11	0	6	16	0	11	0	0			0	44
8:55 AM	1	7	0	7	15	0	11	0	0			0	41
Total Survey	3	361	0	265	368	0	304	5	0			0	1,306

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Bikes	T	R	Bikes	L	R	Bikes	L	T	Bikes	
7:00 AM	0	45	0	38	56	0	30	1	0	0	0	0	170
7:15 AM	1	47	0	32	50	0	39	1	0	0	0	0	170
7:30 AM	0	53	0	41	60	0	46	1	0	0	0	0	201
7:45 AM	0	46	0	44	47	0	49	1	0	0	0	0	187
8:00 AM	1	46	0	32	38	0	24	1	0	0	0	0	142
8:15 AM	0	50	0	28	37	0	34	0	0	0	0	0	149
8:30 AM	0	42	0	26	34	0	51	0	0	0	0	0	153
8:45 AM	1	32	0	24	46	0	31	0	0	0	0	0	134
Total Survey	3	361	0	265	368	0	304	5	0	0	0	0	1,306

Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SE 190th Ave				Southbound SE 190th Ave				Eastbound SE Richey Rd				Westbound SE Richey Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	200	161	361	0	362	367	729	0	173	207	380	0	0	0	0	0	735
%HV	3.5%			2.5%			4.6%			0.0%			0.0%			3.3%	
PHF	0.86			0.85			0.87			0.00			0.90				

Pedestrians

Crosswalk

By Movement	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total	
	L	T	Total	R	T	Total	L	R	Total	L	T	Total		
Volume	1	199	200	156	206	362	168	5	173				0	735
%HV	0.0%	3.5%	NA	3.5%	NA	1.3%	3.4%	2.5%	4.8%	NA	0.0%	4.6%	NA	3.3%
PHF	0.25	0.86		0.86		0.85	0.78	0.85	0.86	0.63	0.87		0.00	0.90

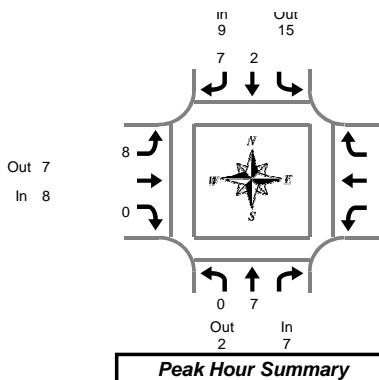
Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Bikes	T	R	Bikes	L	R	Bikes			Bikes	
7:00 AM	1	191	0	155	213	0	164	4	0			0	728
7:15 AM	2	192	0	149	195	0	158	4	0			0	700
7:30 AM	1	195	0	145	182	0	153	3	0			0	679
7:45 AM	1	184	0	130	156	0	158	2	0			0	631
8:00 AM	2	170	0	110	155	0	140	1	0			0	578

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

Heavy Vehicle Summary



SE 190th Ave & SE Richey Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
7:00 AM	0	0	0	0	0	0	0	0	0			0	0
7:05 AM	0	0	0	0	1	1	0	1	1			0	2
7:10 AM	0	0	0	0	0	0	0	1	1			0	1
7:15 AM	0	0	0	1	2	3	1	0	1			0	4
7:20 AM	0	2	2	0	0	0	0	0	0			0	2
7:25 AM	0	1	1	0	1	1	0	0	0			0	2
7:30 AM	0	1	1	0	1	1	0	0	0			0	2
7:35 AM	0	2	2	0	1	1	2	0	2			0	5
7:40 AM	0	0	0	0	1	1	1	0	1			0	2
7:45 AM	0	0	0	0	0	0	1	0	1			0	1
7:50 AM	0	0	0	1	0	1	0	0	0			0	1
7:55 AM	0	1	1	0	0	0	1	0	1			0	2
8:00 AM	0	0	0	0	0	0	0	0	0			0	0
8:05 AM	0	1	1	0	2	2	0	0	0			0	3
8:10 AM	0	2	2	0	0	0	0	0	0			0	2
8:15 AM	0	0	0	0	1	1	1	0	1			0	2
8:20 AM	0	3	3	0	0	0	0	0	0			0	3
8:25 AM	0	0	0	2	1	3	1	0	1			0	4
8:30 AM	0	0	0	0	0	0	0	0	0			0	0
8:35 AM	0	0	0	1	1	2	1	0	1			0	3
8:40 AM	0	0	0	1	0	1	3	0	3			0	4
8:45 AM	0	0	0	0	0	0	1	0	1			0	1
8:50 AM	0	0	0	0	2	2	0	0	0			0	2
8:55 AM	1	0	1	0	1	1	0	0	0			0	2
Total Survey	1	13	14	6	15	21	15	0	15			0	50

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
7:00 AM	0	0	0	0	1	1	2	0	2			0	3
7:15 AM	0	3	3	1	3	4	1	0	1			0	8
7:30 AM	0	3	3	0	3	3	3	0	3			0	9
7:45 AM	0	1	1	1	0	1	2	0	2			0	4
8:00 AM	0	3	3	0	2	2	0	0	0			0	5
8:15 AM	0	3	3	2	2	4	2	0	2			0	9
8:30 AM	0	0	0	2	1	3	4	0	4			0	7
8:45 AM	1	0	1	0	3	3	1	0	1			0	5
Total Survey	1	13	14	6	15	21	15	0	15			0	50

Heavy Vehicle Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	7	2	9	9	15	24	8	7	15	0	0	0	24
PHF	0.44		0.56			0.50			0.50			0.67	

By Movement	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	L	T	Total	T	R	Total	L	R	Total			Total	
Volume	0	7	7	2	7	9	8	0	8			0	24
PHF	0.00	0.44	0.44	0.50	0.58	0.56	0.50	0.00	0.50			0.00	0.67

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
7:00 AM	0	7	7	2	7	9	8	0	8			0	24
7:15 AM	0	10	10	2	8	10	6	0	6			0	26
7:30 AM	0	10	10	3	7	10	7	0	7			0	27
7:45 AM	0	7	7	5	5	10	8	0	8			0	25
8:00 AM	1	6	7	4	8	12	7	0	7			0	26

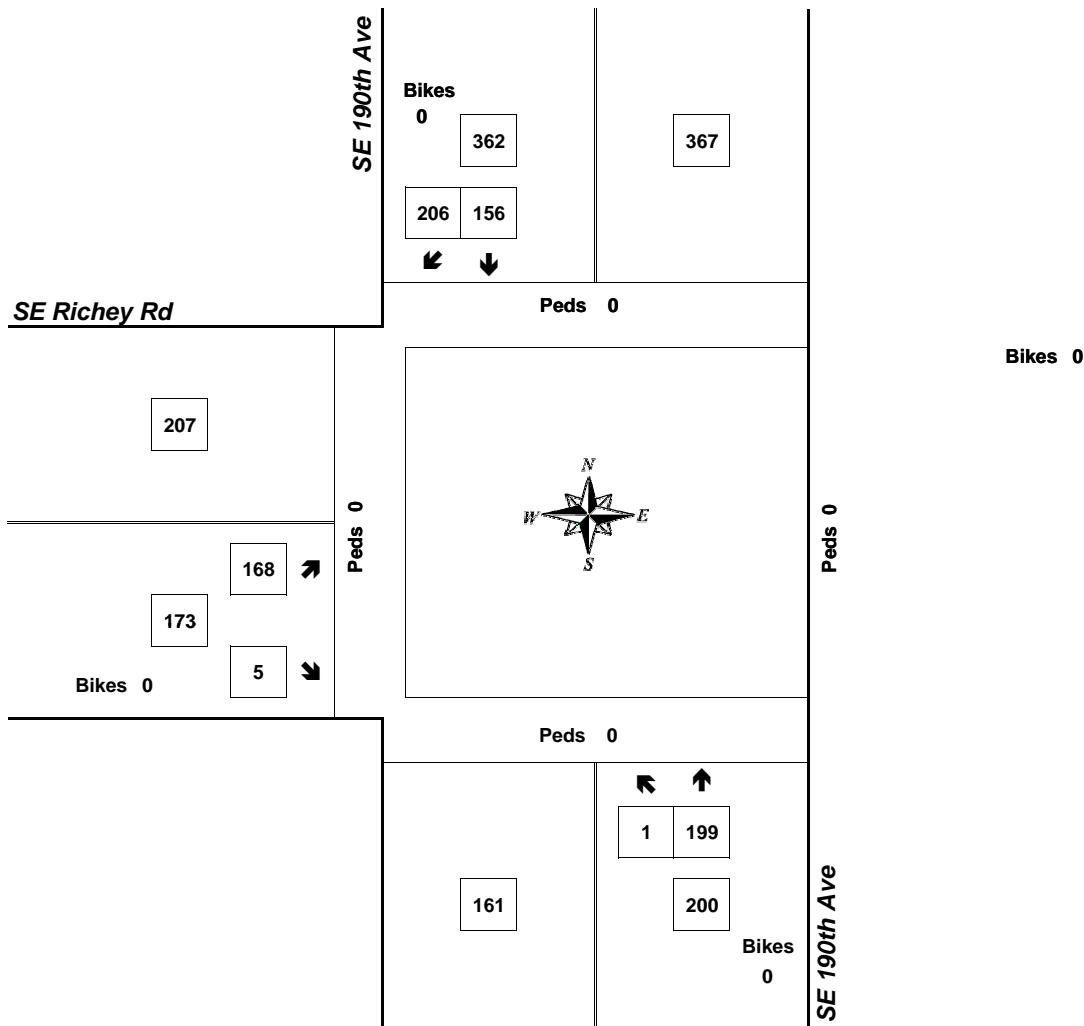
Peak Hour Summary



Clay Carney
(503) 833-2740

SE 190th Ave & SE Richey Rd

7:05 AM to 8:05 AM
Tuesday, May 07, 2019



Approach	PHF	HV%	Volume
EB	0.87	4.6%	173
WB	0.00	0.0%	0
NB	0.86	3.5%	200
SB	0.85	2.5%	362
Intersection	0.90	3.3%	735

Count Period: 7:00 AM to 9:00 AM

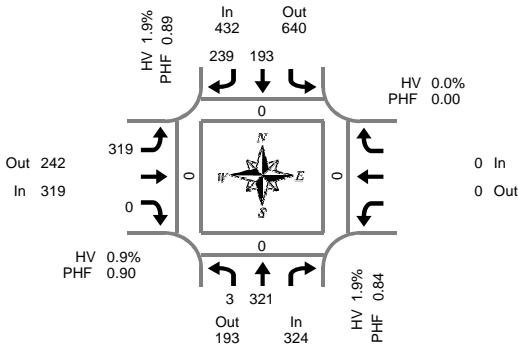
Total Vehicle Summary



Clay Carney
(503) 833-2740

SE 190th Ave & SE Richey Rd

**Tuesday, May 07, 2019
4:00 PM to 6:00 PM**



**Peak Hour Summary
4:50 PM to 5:50 PM**

5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Bikes	T	R	Bikes	L	R	Bikes			Bikes	
4:00 PM	0	17	0	18	10	0	31	0	0			0	76
4:05 PM	0	30	0	12	10	0	30	0	0			0	82
4:10 PM	0	23	0	19	23	0	18	0	0			0	83
4:15 PM	0	31	0	14	11	0	25	0	0			0	81
4:20 PM	1	32	0	14	20	0	24	0	0			0	91
4:25 PM	0	16	0	19	23	0	23	0	0			0	81
4:30 PM	0	18	0	23	20	0	21	0	0			0	82
4:35 PM	0	29	0	18	21	0	22	0	0			0	90
4:40 PM	0	19	0	12	20	0	20	0	0			0	71
4:45 PM	0	34	0	11	15	0	19	0	0			0	79
4:50 PM	0	23	0	21	19	0	24	0	0			0	87
4:55 PM	0	26	0	13	14	0	24	0	0			0	77
5:00 PM	0	31	0	16	21	0	24	0	0			0	92
5:05 PM	0	21	0	17	19	0	28	0	0			0	85
5:10 PM	0	27	0	25	22	0	27	0	0			0	101
5:15 PM	0	32	0	12	26	0	30	0	0			0	100
5:20 PM	2	35	0	17	20	0	32	0	0			0	106
5:25 PM	0	26	0	15	19	0	24	0	0			0	84
5:30 PM	0	26	0	10	19	0	20	0	0			0	75
5:35 PM	1	27	0	16	16	0	23	0	0			0	83
5:40 PM	0	25	0	14	22	0	33	0	0			0	94
5:45 PM	0	22	0	17	22	0	30	0	0			0	91
5:50 PM	0	29	0	18	15	0	11	0	0			0	73
5:55 PM	0	26	0	9	17	0	20	0	0			0	72
Total Survey	4	625	0	380	444	0	583	0	0			0	2,036

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Bikes	T	R	Bikes	L	R	Bikes			Bikes	
4:00 PM	0	70	0	49	43	0	79	0	0			0	241
4:15 PM	1	79	0	47	54	0	72	0	0			0	253
4:30 PM	0	66	0	53	61	0	63	0	0			0	243
4:45 PM	0	83	0	45	48	0	67	0	0			0	243
5:00 PM	0	79	0	58	62	0	79	0	0			0	278
5:15 PM	2	93	0	44	65	0	86	0	0			0	290
5:30 PM	1	78	0	40	57	0	76	0	0			0	252
5:45 PM	0	77	0	44	54	0	61	0	0			0	236
Total Survey	4	625	0	380	444	0	583	0	0			0	2,036

Peak Hour Summary

4:50 PM to 5:50 PM

By Approach	Northbound SE 190th Ave				Southbound SE 190th Ave				Eastbound SE Richey Rd				Westbound SE Richey Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
	Volume	324	193	517	0	432	640	1,072	0	319	242	561	0	0	0	0	0
%HV	1.9%			1.9%			0.9%			0.0%			0.0%			1.6%	
PHF	0.84			0.89			0.90			0.00			0.00			0.88	

Pedestrians Crosswalk

By Movement	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	L	T	Total	T	R	Total	L	R	Total	L	R	Total	
Volume	3	321	324	193	239	432	319	0	319	0	0	0	1,075
%HV	0.0%	1.9%	NA	1.9%	NA	1.0%	2.5%	1.9%	0.9%	NA	0.0%	0.9%	1.6%
PHF	0.38	0.85		0.84		0.83	0.88	0.89	0.90	0.00	0.90	0.00	0.88

Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total	
	L	T	Bikes	T	R	Bikes	L	R	Bikes			Bikes		
4:00 PM	1	298	0	194	206	0	281		0	0			0	980
4:15 PM	1	307	0	203	225	0	281		0	0			0	1,017
4:30 PM	2	321	0	200	236	0	295		0	0			0	1,054
4:45 PM	3	333	0	187	232	0	308		0	0			0	1,063
5:00 PM	3	327	0	186	238	0	302		0	0			0	1,056

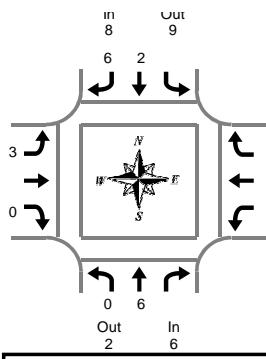
Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



SE 190th Ave & SE Richey Rd

Tuesday, May 07, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:50 PM to 5:50 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
4:00 PM	0	1	1	1	1	2	1	0	1			0	4
4:05 PM	0	0	0	0	1	1	2	0	2			0	3
4:10 PM	0	2	2	0	0	0	0	0	0			0	2
4:15 PM	0	0	0	2	1	3	1	0	1			0	4
4:20 PM	0	0	0	1	4	5	0	0	0			0	5
4:25 PM	0	0	0	0	1	1	1	0	1			0	2
4:30 PM	0	0	0	0	0	0	0	0	0			0	0
4:35 PM	0	0	0	0	0	0	1	0	1			0	1
4:40 PM	0	0	0	0	0	0	0	0	0			0	0
4:45 PM	0	0	0	0	0	0	0	0	0			0	0
4:50 PM	0	1	1	0	0	0	0	0	0			0	1
4:55 PM	0	1	1	0	0	0	2	0	2			0	3
5:00 PM	0	0	0	0	3	3	0	0	0			0	3
5:05 PM	0	2	2	0	0	0	0	0	0			0	2
5:10 PM	0	0	0	0	0	0	0	0	0			0	0
5:15 PM	0	0	0	0	1	1	1	0	1			0	2
5:20 PM	0	0	0	0	0	0	0	0	0			0	0
5:25 PM	0	0	0	0	0	0	0	0	0			0	0
5:30 PM	0	1	1	1	1	2	0	0	0			0	3
5:35 PM	0	0	0	0	0	0	0	0	0			0	0
5:40 PM	0	0	0	0	1	1	0	0	0			0	1
5:45 PM	0	1	1	1	0	1	0	0	0			0	2
5:50 PM	0	0	0	1	0	1	0	0	0			0	1
5:55 PM	0	1	1	0	0	0	0	0	0			0	1
Total Survey	0	10	10	7	14	21	9	0	9			0	40

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
4:00 PM	0	3	3	1	2	3	3	0	3			0	9
4:15 PM	0	0	0	3	6	9	2	0	2			0	11
4:30 PM	0	0	0	0	0	0	1	0	1			0	1
4:45 PM	0	2	2	0	0	0	2	0	2			0	4
5:00 PM	0	2	2	0	3	3	0	0	0			0	5
5:15 PM	0	0	0	0	1	1	1	0	1			0	2
5:30 PM	0	1	1	1	2	3	0	0	0			0	4
5:45 PM	0	2	2	2	0	2	0	0	0			0	4
Total Survey	0	10	10	7	14	21	9	0	9			0	40

Heavy Vehicle Peak Hour Summary

4:50 PM to 5:50 PM

By Approach	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	6	2	8	8	9	17	3	6	9	0	0	0	17
PHF	0.50		0.67			0.38			0.00			0.53	

By Movement	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	L	T	Total	T	R	Total	L	R	Total			Total	
Volume	0	6	6	2	6	8	3	0	3			0	17
PHF	0.00	0.50	0.50	0.50	0.50	0.67	0.38		0.00	0.38		0.00	0.53

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE 190th Ave			Southbound SE 190th Ave			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	L	T	Total	T	R	Total	L	R	Total			Total	
4:00 PM	0	5	5	4	8	12	8	0	8			0	25
4:15 PM	0	4	4	3	9	12	5	0	5			0	21
4:30 PM	0	4	4	0	4	4	4	0	4			0	12
4:45 PM	0	5	5	1	6	7	3	0	3			0	15
5:00 PM	0	5	5	3	6	9	1	0	1			0	15

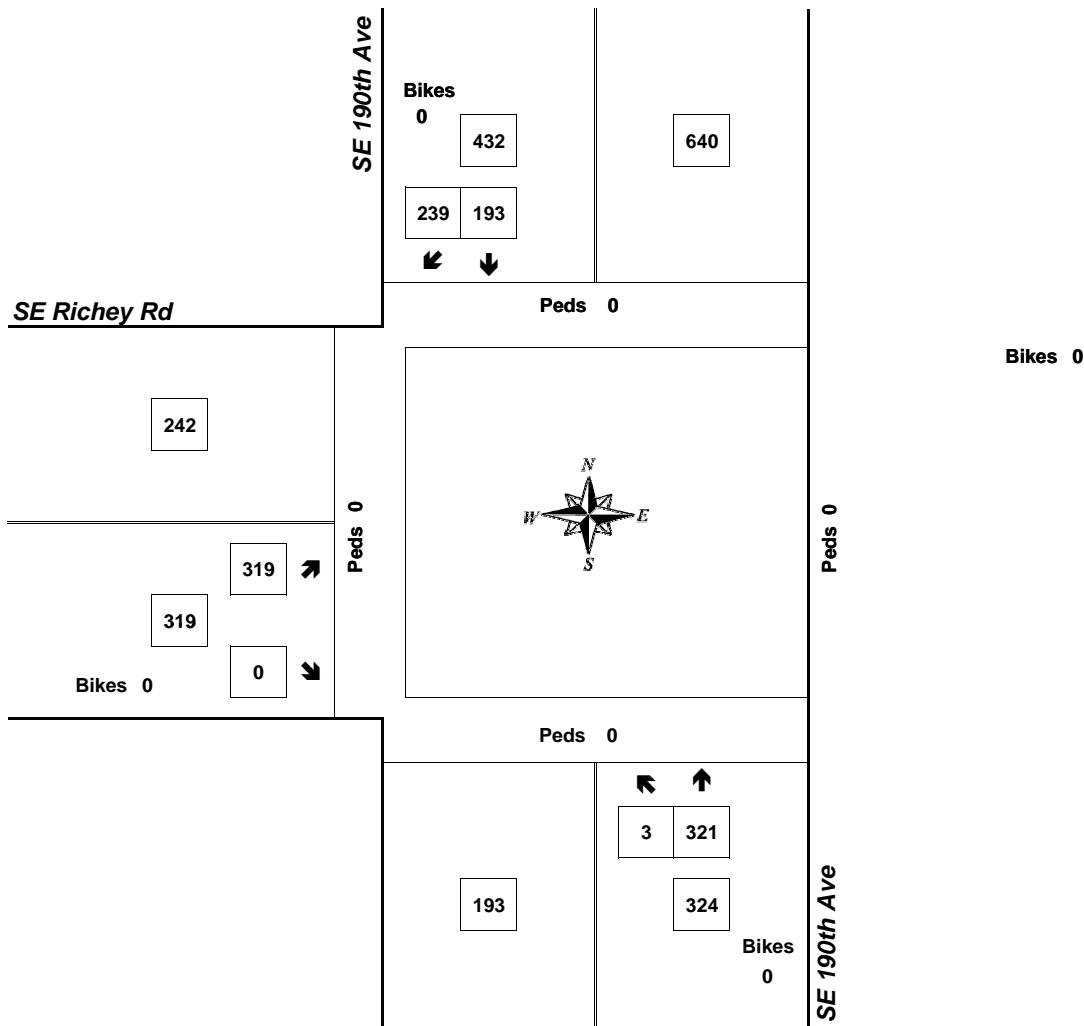
Peak Hour Summary



Clay Carney
(503) 833-2740

SE 190th Ave & SE Richey Rd

4:50 PM to 5:50 PM
Tuesday, May 07, 2019



Approach	PHF	HV%	Volume
EB	0.90	0.9%	319
WB	0.00	0.0%	0
NB	0.84	1.9%	324
SB	0.89	1.9%	432
Intersection	0.88	1.6%	1,075

Count Period: 4:00 PM to 6:00 PM

Total Vehicle Summary



Clay Carney
(503) 833-2740

Clay Carney

Clay Caney
(503) 833-2740

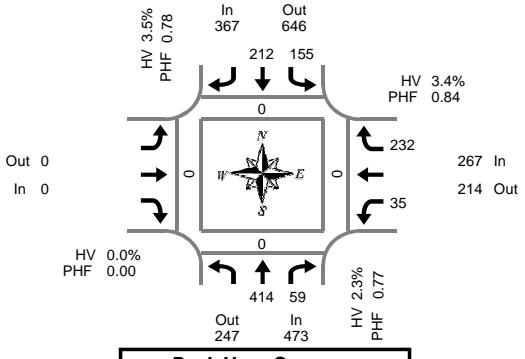
Clay Carney
(503) 822-8740

(503) 833-2740

SE Foster Rd & SE Richey Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM



Peak Hour Summary
7:05 AM to 8:05 AM

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
7:00 AM	24	3	0	7	7	0			0	2	26	0	69
7:05 AM	35	0	0	9	12	0			0	5	16	0	77
7:10 AM	32	2	0	16	14	0			0	2	21	0	87
7:15 AM	33	6	0	12	15	0			0	2	19	0	87
7:20 AM	38	2	0	15	12	0			0	2	21	0	90
7:25 AM	32	8	0	10	12	0			0	2	17	0	81
7:30 AM	37	7	0	15	21	0			0	3	22	0	105
7:35 AM	52	10	0	12	26	0			0	5	20	0	125
7:40 AM	38	9	0	11	30	0			0	5	24	0	117
7:45 AM	34	4	0	16	22	0			0	2	16	0	94
7:50 AM	33	5	0	11	18	0			0	2	20	0	89
7:55 AM	26	5	0	18	13	0			0	2	14	0	78
8:00 AM	24	1	0	10	17	0			0	3	22	0	77
8:05 AM	26	1	0	10	17	0			0	2	14	0	70
8:10 AM	16	2	0	1	13	0			0	1	12	0	45
8:15 AM	14	3	0	15	18	0			0	1	17	0	68
8:20 AM	35	3	0	9	11	0			0	2	14	0	74
8:25 AM	21	5	0	10	20	0			0	2	20	0	78
8:30 AM	22	2	0	14	13	0			0	3	14	0	68
8:35 AM	27	3	0	10	11	0			0	1	16	0	68
8:40 AM	25	3	0	17	4	0			0	3	16	0	68
8:45 AM	24	3	0	7	14	0			0	0	12	0	60
8:50 AM	19	4	0	9	10	0			0	2	22	0	66
8:55 AM	18	2	0	9	14	0			0	1	10	0	54
Total Survey	685	93	0	273	364	0			0	55	425	0	1,895

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
7:00 AM	91	5	0	32	33	0			0	9	63	0	233
7:15 AM	103	16	0	37	39	0			0	6	57	0	258
7:30 AM	127	26	0	38	77	0			0	13	66	0	347
7:45 AM	93	14	0	45	53	0			0	6	50	0	261
8:00 AM	66	4	0	21	47	0			0	6	48	0	192
8:15 AM	70	11	0	34	49	0			0	5	51	0	220
8:30 AM	74	8	0	41	28	0			0	7	46	0	204
8:45 AM	61	9	0	25	38	0			0	3	44	0	180
Total Survey	685	93	0	273	364	0			0	55	425	0	1,895

Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SE Foster Rd				Southbound SE Foster Rd				Eastbound SE Richey Rd				Westbound SE Richey Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	473	247	720	0	367	646	1,013	0	0	0	0	0	267	214	481	0	1,107
%HV	2.3%			3.5%			0.0%			3.4%			3.0%				
PHF	0.77			0.78			0.00			0.84			0.80				

Pedestrians

Crosswalk

By Movement	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total			
	T	R	Total	L	T	Total			Total	L	R	Total				
Volume	414	59	473	155	212	367			0	35	232	267	1,107			
%HV	NA	2.2%	3.4%	2.3%	2.6%	4.2%	NA	3.5%	NA	NA	0.0%	5.7%	NA	3.0%	3.4%	3.0%
PHF	0.81	0.57	0.77	0.86	0.68	0.78			0.00	0.67	0.88	0.84	0.80			

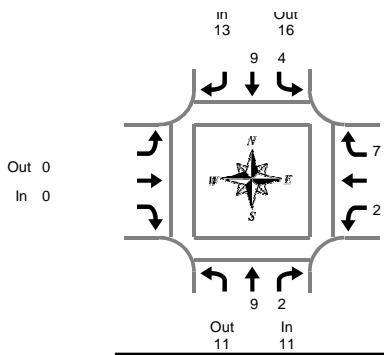
Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
7:00 AM	414	61	0	152	202	0			0	34	236	0	1,099
7:15 AM	389	60	0	141	216	0			0	31	221	0	1,058
7:30 AM	356	55	0	138	226	0			0	30	215	0	1,020
7:45 AM	303	37	0	141	177	0			0	24	195	0	877
8:00 AM	271	32	0	121	162	0			0	21	189	0	796

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

Heavy Vehicle Summary



SE Foster Rd & SE Richey Rd

Tuesday, May 07, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total	
7:00 AM	0	0	0	0	0	0			0	0	0	0	0
7:05 AM	2	0	2	0	0	0			0	0	1	1	3
7:10 AM	0	0	0	1	0	1			0	0	0	0	1
7:15 AM	0	1	1	0	0	0			0	0	0	0	1
7:20 AM	2	0	2	0	1	1			0	0	0	0	3
7:25 AM	0	0	0	0	4	4			0	0	0	0	4
7:30 AM	0	0	0	0	4	4			0	0	1	1	5
7:35 AM	2	0	2	1	0	1			0	0	0	0	3
7:40 AM	0	0	0	1	0	1			0	0	3	3	4
7:45 AM	1	0	1	0	0	0			0	0	0	0	1
7:50 AM	2	0	2	0	0	0			0	0	1	1	3
7:55 AM	0	1	1	1	0	1			0	2	0	2	4
8:00 AM	0	0	0	0	0	0			0	0	1	1	1
8:05 AM	0	0	0	1	0	1			0	0	2	2	3
8:10 AM	0	0	0	0	1	1			0	0	0	0	1
8:15 AM	0	0	0	2	2	4			0	0	0	0	4
8:20 AM	0	1	1	0	0	0			0	0	1	1	2
8:25 AM	0	1	1	2	2	4			0	0	0	0	5
8:30 AM	1	0	1	1	0	1			0	0	1	1	3
8:35 AM	0	0	0	3	0	3			0	0	0	0	3
8:40 AM	2	0	2	1	0	1			0	0	1	1	4
8:45 AM	1	0	1	1	1	2			0	0	1	1	4
8:50 AM	1	0	1	0	0	0			0	0	1	1	2
8:55 AM	1	0	1	0	1	1			0	0	1	1	3
Total Survey	15	4	19	15	16	31			0	2	15	17	67

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total	
7:00 AM	2	0	2	1	0	1			0	0	1	1	4
7:15 AM	2	1	3	0	5	5			0	0	0	0	8
7:30 AM	2	0	2	2	4	6			0	0	4	4	12
7:45 AM	3	1	4	1	0	1			0	2	1	3	8
8:00 AM	0	0	0	1	1	2			0	0	3	3	5
8:15 AM	0	2	2	4	4	8			0	0	1	1	11
8:30 AM	3	0	3	5	0	5			0	0	2	2	10
8:45 AM	3	0	3	1	2	3			0	0	3	3	9
Total Survey	15	4	19	15	16	31			0	2	15	17	67

Heavy Vehicle Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	11	11	22	13	16	29	0	0	0	9	6	15	33
PHF	0.69		0.36			0.00			0.56			0.69	

By Movement	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	T	R	Total	L	T	Total			Total	L	R	Total	
Volume	9	2	11	4	9	13			0	2	7	9	33
PHF	0.75	0.50	0.69	0.50	0.25	0.36			0.00	0.25	0.44	0.56	0.69

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total	
7:00 AM	9	2	11	4	9	13			0	2	6	8	32
7:15 AM	7	2	9	4	10	14			0	2	8	10	33
7:30 AM	5	3	8	8	9	17			0	2	9	11	36
7:45 AM	6	3	9	11	5	16			0	2	7	9	34
8:00 AM	6	2	8	11	7	18			0	0	9	9	35

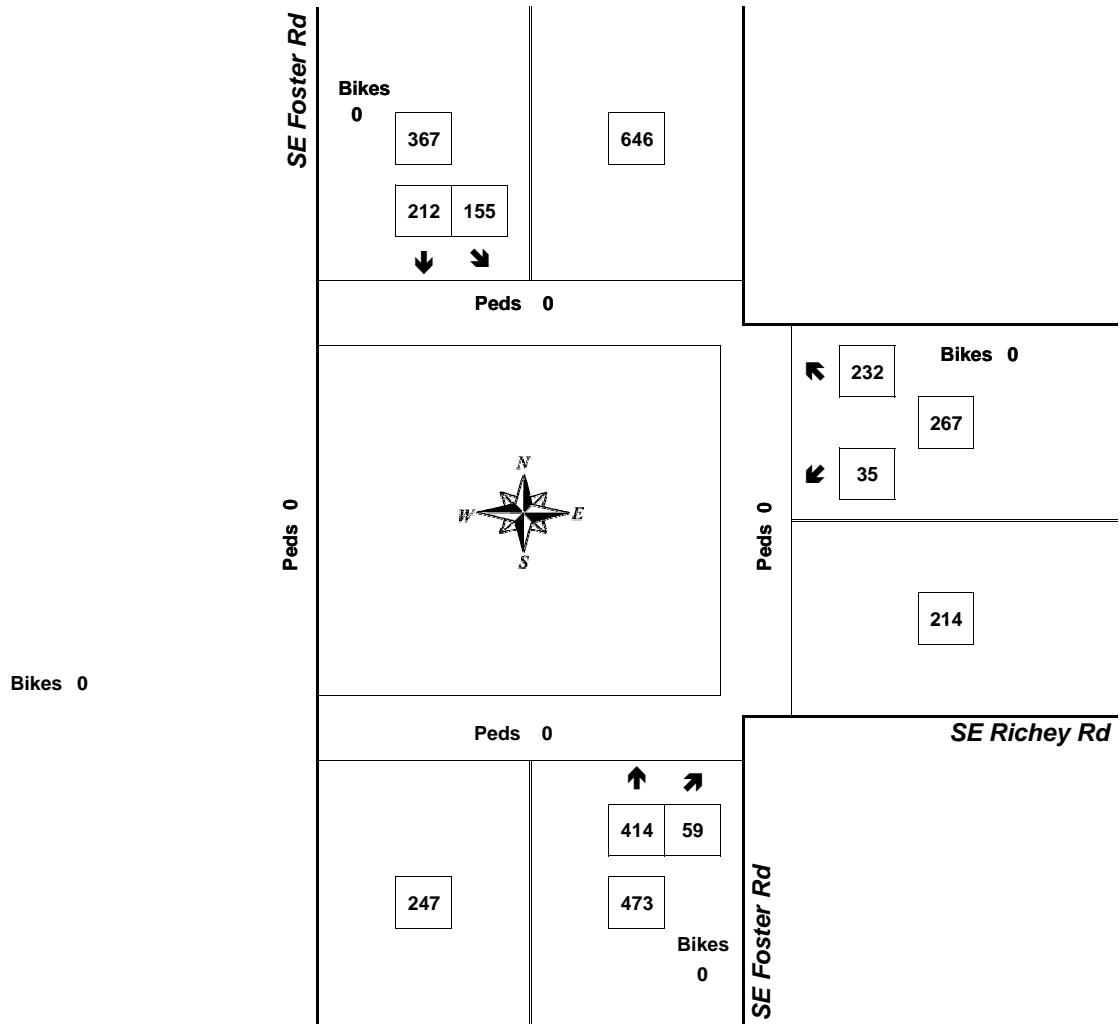
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Foster Rd & SE Richey Rd

7:05 AM to 8:05 AM
Tuesday, May 07, 2019



Count Period: 7:00 AM to 9:00 AM

Total Vehicle Summary

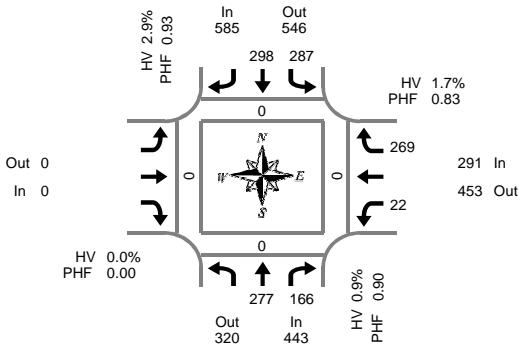


Clay Carney
(503) 833-2740

SE Foster Rd & SE Richey Rd

Tuesday, May 07, 2019

4:00 PM to 6:00 PM



Peak Hour Summary
4:45 PM to 5:45 PM

5-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
4:00 PM	36	14	0	27	29	0			0	2	15	0	123
4:05 PM	25	10	0	24	24	0			0	2	12	0	97
4:10 PM	25	8	0	17	21	0			0	4	17	0	92
4:15 PM	34	11	0	20	22	0			0	2	20	0	109
4:20 PM	20	9	0	30	19	0			0	3	16	0	97
4:25 PM	28	12	0	22	29	0			0	1	19	0	111
4:30 PM	19	8	0	19	28	1			0	5	22	0	101
4:35 PM	27	4	0	27	13	1			0	3	20	0	94
4:40 PM	22	16	0	20	25	1			0	4	21	0	108
4:45 PM	23	11	0	26	29	0			0	3	23	0	115
4:50 PM	32	9	0	30	18	0			0	0	13	0	102
4:55 PM	21	16	0	16	28	0			0	1	29	0	111
5:00 PM	18	18	0	16	20	0			0	3	13	0	88
5:05 PM	16	8	0	33	23	0			0	1	27	0	108
5:10 PM	28	18	0	29	17	0			0	3	18	0	113
5:15 PM	27	9	0	25	27	0			0	0	29	0	117
5:20 PM	24	17	0	32	27	0			0	2	29	0	131
5:25 PM	17	15	0	11	27	0			0	6	22	0	98
5:30 PM	25	10	0	18	35	0			0	1	23	0	112
5:35 PM	21	17	0	19	27	0			0	1	23	0	108
5:40 PM	25	18	0	32	20	0			0	1	20	0	116
5:45 PM	32	16	0	23	16	0			0	5	23	0	115
5:50 PM	24	3	0	13	35	0			0	2	21	0	98
5:55 PM	31	11	0	20	26	0			0	4	15	0	107
Total Survey	600	288	0	549	585	3			0	59	490	0	2,571

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
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0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
4:00 PM	86	32	0	68	74	0			0	8	44	0	312
4:15 PM	82	32	0	72	70	0			0	6	55	0	317
4:30 PM	68	28	0	66	66	3			0	12	63	0	303
4:45 PM	76	36	0	72	75	0			0	4	65	0	328
5:00 PM	62	44	0	78	60	0			0	7	58	0	309
5:15 PM	68	41	0	68	81	0			0	8	80	0	346
5:30 PM	71	45	0	69	82	0			0	3	66	0	336
5:45 PM	87	30	0	56	77	0			0	11	59	0	320
Total Survey	600	288	0	549	585	3			0	59	490	0	2,571

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

Peak Hour Summary 4:45 PM to 5:45 PM

By Approach	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	443	320	763	0	585	546	1,131	0	0	0	0	0	1,319
%HV	0.9%				2.9%			0.0%				1.7%	2.0%
PHF	0.90				0.93			0.00			0.83		0.91

By Movement	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	T	R	Total	L	T	Total			Bikes	L	R	Total	
Volume	277	166	443	287	298	585			0	22	269	291	1,319
%HV	NA	1.1%	0.6%	0.9%	1.7%	4.0%	NA	2.9%	NA	NA	NA	1.9%	1.7%
PHF	0.88	0.92	0.90	0.82	0.84	0.93			0.00	0.61	0.84	0.83	0.91

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
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0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

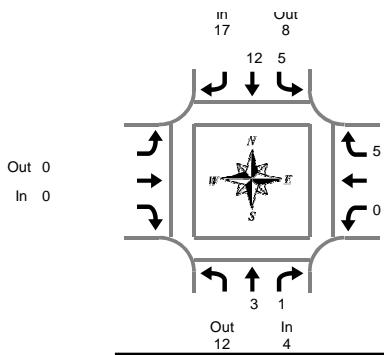
Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Interval Total
	T	R	Bikes	L	T	Bikes			Bikes	L	R	Bikes	
4:00 PM	312	128	0	278	285	3			0	30	227	0	1,260
4:15 PM	288	140	0	288	271	3			0	29	241	0	1,257
4:30 PM	274	149	0	284	282	3			0	31	266	0	1,286
4:45 PM	277	166	0	287	298	0			0	22	269	0	1,319
5:00 PM	288	160	0	271	300	0			0	29	263	0	1,311

Pedestrians Crosswalk			
North	South	East	West

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Heavy Vehicle Summary



SE Foster Rd & SE Richey Rd

Tuesday, May 07, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:45 PM to 5:45 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd				Southbound SE Foster Rd				Eastbound SE Richey Rd				Westbound SE Richey Rd				Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total					
4:00 PM	2	1	3	1	1	2			0	0	0	0	0	0	0	0	5
4:05 PM	1	1	2	1	0	1			0	0	1	1	0	0	1	1	4
4:10 PM	1	0	1	1	0	1			0	0	2	2	0	0	0	0	4
4:15 PM	2	0	2	1	0	1			0	0	0	0	0	0	0	0	3
4:20 PM	0	0	0	0	0	0			0	0	2	2	0	0	2	2	2
4:25 PM	1	0	1	1	2	3			0	0	2	2	0	0	2	2	6
4:30 PM	1	0	1	0	0	0			0	0	0	0	0	0	0	0	1
4:35 PM	1	0	1	1	0	1			0	0	0	0	0	0	0	0	2
4:40 PM	0	1	1	0	0	0			0	0	0	0	0	0	0	0	1
4:45 PM	1	0	1	0	2	2			0	0	0	0	0	0	0	0	3
4:50 PM	0	0	0	0	1	1			0	0	0	0	0	0	0	0	1
4:55 PM	0	1	1	1	3	4			0	0	0	0	0	0	0	0	5
5:00 PM	0	0	0	0	0	0			0	0	1	1	0	0	1	1	1
5:05 PM	0	0	0	1	0	1			0	0	2	2	0	0	2	2	3
5:10 PM	0	0	0	0	0	0			0	0	0	0	0	0	0	0	0
5:15 PM	1	0	1	1	2	3			0	0	1	1	0	0	1	1	5
5:20 PM	0	0	0	0	0	0			0	0	0	0	0	0	0	0	0
5:25 PM	0	0	0	0	0	0			0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	1	1	2			0	0	0	0	0	0	0	0	2
5:35 PM	0	0	0	0	0	0			0	0	1	1	0	0	1	1	1
5:40 PM	1	0	1	1	3	4			0	0	0	0	0	0	0	0	5
5:45 PM	0	0	0	0	0	0			0	0	1	1	0	0	1	1	1
5:50 PM	0	0	0	0	0	0			0	0	0	0	0	0	0	0	0
5:55 PM	2	1	3	0	0	0			0	0	0	0	0	0	0	0	3
Total Survey	14	5	19	11	15	26			0	0	13	13	0	0	13	13	58

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd				Southbound SE Foster Rd				Eastbound SE Richey Rd				Westbound SE Richey Rd				Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total					
4:00 PM	4	2	6	3	1	4			0	0	3	3	0	0	3	3	13
4:15 PM	3	0	3	2	2	4			0	0	4	4	0	0	4	4	11
4:30 PM	2	1	3	1	0	1			0	0	0	0	0	0	0	0	4
4:45 PM	1	1	2	1	6	7			0	0	0	0	0	0	0	0	9
5:00 PM	0	0	0	1	0	1			0	0	3	3	0	0	3	3	4
5:15 PM	1	0	1	1	2	3			0	0	1	1	0	0	1	1	5
5:30 PM	1	0	1	2	4	6			0	0	1	1	0	0	1	1	8
5:45 PM	2	1	3	0	0	0			0	0	1	1	0	0	1	1	4
Total Survey	14	5	19	11	15	26			0	0	13	13	0	0	13	13	58

Heavy Vehicle Peak Hour Summary

4:45 PM to 5:45 PM

By Approach	Northbound SE Foster Rd			Southbound SE Foster Rd			Eastbound SE Richey Rd			Westbound SE Richey Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	4	12	16	17	8	25	0	0	0	5	6	11	26
PHF	0.50		0.61			0.00			0.42			0.72	

By Movement	Northbound SE Foster Rd				Southbound SE Foster Rd				Eastbound SE Richey Rd				Westbound SE Richey Rd				Total
	T	R	Total	L	T	Total			Total	L	R	Total					
Volume	3	1	4	5	12	17			0	0	5	5	0	0	0	0	26
PHF	0.75	0.25	0.50	0.63	0.50	0.61			0.00	0.00	0.42	0.42	0.00	0.00	0.00	0.00	0.72

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Foster Rd				Southbound SE Foster Rd				Eastbound SE Richey Rd				Westbound SE Richey Rd				Interval Total
	T	R	Total	L	T	Total			Total	L	R	Total					
4:00 PM	10	4	14	7	9	16			0	0	7	7	0	0	0	0	37
4:15 PM	6	2	8	5	8	13			0	0	7	7	0	0	0	0	28
4:30 PM	4	2	6	4	8	12			0	0	4	4	0	0	0	0	22
4:45 PM	3	1	4	5	12	17			0	0	5	5	0	0	0	0	26
5:00 PM	4	1	5	4	6	10			0	0	6	6	0	0	0	0	21

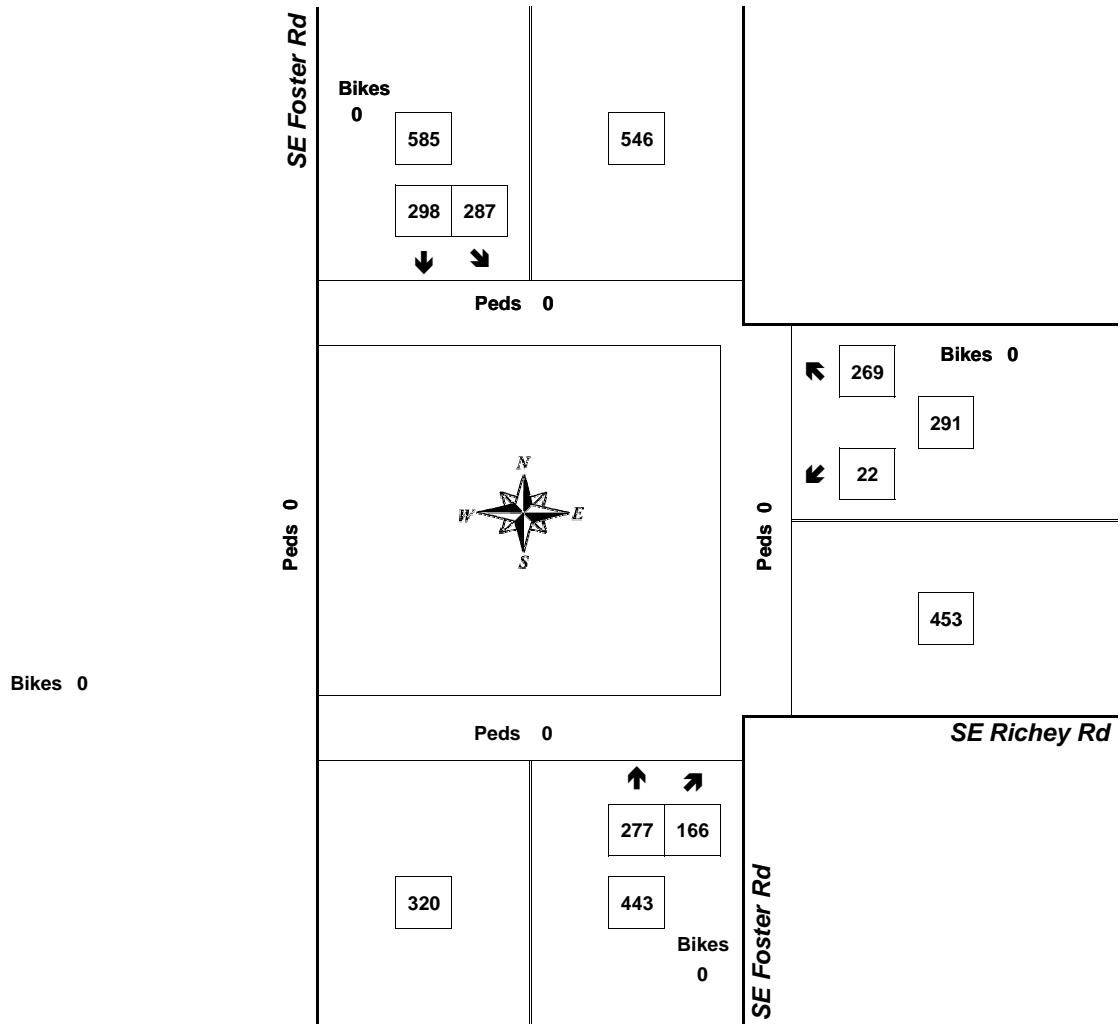
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Foster Rd & SE Richey Rd

4:45 PM to 5:45 PM
Tuesday, May 07, 2019



Approach	PHF	HV%	Volume
EB	0.00	0.0%	0
WB	0.83	1.7%	291
NB	0.90	0.9%	443
SB	0.93	2.9%	585
Intersection	0.91	2.0%	1,319

Count Period: 4:00 PM to 6:00 PM

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

04/21/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	191	464	82	63	493	125	204	767	45	68	263	140
Future Volume (vph)	191	464	82	63	493	125	204	767	45	68	263	140
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.98		1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1719	3351		1719	3321		1770	3505		1687	3374	1436
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1719	3351		1719	3321		1770	3505		1687	3374	1436
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	208	504	89	68	536	136	222	834	49	74	286	152
RTOR Reduction (vph)	0	19	0	0	29	0	0	6	0	0	0	119
Lane Group Flow (vph)	208	574	0	68	643	0	222	877	0	74	286	33
Confl. Peds. (#/hr)			5			5			10			28
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	2%	2%	2%	7%	7%	7%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	14.7	22.8		10.1	18.2		12.6	24.7		4.8	17.2	17.2
Effective Green, g (s)	14.7	22.8		10.1	18.2		12.6	24.7		4.8	17.2	17.2
Actuated g/C Ratio	0.18	0.29		0.13	0.23		0.16	0.31		0.06	0.21	0.21
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	315	955		217	755		278	1082		101	725	308
v/s Ratio Prot	c0.12	0.17		0.04	c0.19		c0.13	c0.25		0.04	0.08	
v/s Ratio Perm												0.02
v/c Ratio	0.66	0.60		0.31	0.85		0.80	0.81		0.73	0.39	0.11
Uniform Delay, d1	30.3	24.7		31.8	29.6		32.5	25.5		37.0	26.9	25.2
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	10.4	2.8		3.7	11.7		14.7	4.7		23.7	0.4	0.2
Delay (s)	40.7	27.5		35.5	41.3		47.2	30.2		60.7	27.3	25.4
Level of Service	D	C		D	D		D	C		E	C	C
Approach Delay (s)		30.9			40.8			33.6			31.6	
Approach LOS		C			D			C			C	
Intersection Summary												
HCM 2000 Control Delay		34.3										C
HCM 2000 Volume to Capacity ratio		0.82										
Actuated Cycle Length (s)		80.0										17.6
Intersection Capacity Utilization		69.8%										C
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

04/21/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	191	464	82	63	493	125	204	767	45	68	263	140
Future Volume (veh/h)	191	464	82	63	493	125	204	767	45	68	263	140
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.96
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1826	1826	1826	1826	1826	1826	1870	1870	1870	1796	1796	1796
Adj Flow Rate, veh/h	208	504	89	68	536	136	222	834	49	74	286	152
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	5	5	5	5	5	5	2	2	2	7	7	7
Cap, veh/h	239	1060	186	139	828	209	261	974	57	94	662	282
Arrive On Green	0.14	0.36	0.36	0.08	0.30	0.30	0.15	0.29	0.29	0.05	0.19	0.19
Sat Flow, veh/h	1739	2947	518	1739	2740	692	1781	3408	200	1711	3413	1456
Grp Volume(v), veh/h	208	296	297	68	339	333	222	435	448	74	286	152
Grp Sat Flow(s), veh/h/ln	1739	1735	1730	1739	1735	1697	1781	1777	1832	1711	1706	1456
Q Serve(g_s), s	9.4	10.5	10.6	3.0	13.5	13.7	9.7	18.5	18.5	3.4	5.9	7.5
Cycle Q Clear(g_c), s	9.4	10.5	10.6	3.0	13.5	13.7	9.7	18.5	18.5	3.4	5.9	7.5
Prop In Lane	1.00		0.30	1.00		0.41	1.00		0.11	1.00		1.00
Lane Grp Cap(c), veh/h	239	624	622	139	524	513	261	508	523	94	662	282
V/C Ratio(X)	0.87	0.47	0.48	0.49	0.65	0.65	0.85	0.86	0.86	0.79	0.43	0.54
Avail Cap(c_a), veh/h	239	624	622	139	524	513	292	566	584	124	789	337
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.8	19.8	19.8	35.2	24.2	24.2	33.3	27.0	27.0	37.4	28.4	29.0
Incr Delay (d2), s/veh	32.4	2.6	2.6	11.8	6.0	6.3	19.1	11.4	11.1	21.7	0.4	1.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	5.9	4.3	4.4	1.7	6.0	5.9	5.4	9.0	9.2	1.9	2.3	2.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	66.2	22.4	22.4	47.0	30.3	30.5	52.4	38.4	38.1	59.1	28.8	30.6
LnGrp LOS	E	C	C	D	C	C	D	D	D	E	C	C
Approach Vol, veh/h		801			740			1105			512	
Approach Delay, s/veh		33.8			31.9			41.1			33.7	
Approach LOS		C			C			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.4	33.6	8.4	27.7	15.0	29.0	15.7	20.3				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	6.4	24.7	5.8	25.5	11.0	20.1	13.1	* 19				
Max Q Clear Time (g_c+l1), s	5.0	12.6	5.4	20.5	11.4	15.7	11.7	9.5				
Green Ext Time (p_c), s	0.0	2.7	0.0	2.3	0.0	1.6	0.1	1.5				
Intersection Summary												
HCM 6th Ctrl Delay		35.9										
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection

Intersection Delay, s/veh 17.1

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	28	19	0	93	62	206	0	308	69	126	282	36
Future Vol, veh/h	28	19	0	93	62	206	0	308	69	126	282	36
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	1	1	1	4	4	4	4	4	4
Mvmt Flow	30	21	0	101	67	224	0	335	75	137	307	39
Number of Lanes	0	1	0	0	1	1	0	1	1	1	1	0
Approach												
Opposing Approach	WB			WB			SB			SB		
Opposing Lanes	2			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			2			1		
HCM Control Delay	12.4			13.9			19.1			18.6		
HCM LOS	B			B			C			C		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	0%	0%	60%	60%	0%	100%	0%
Vol Thru, %	100%	0%	40%	40%	0%	0%	89%
Vol Right, %	0%	100%	0%	0%	100%	0%	11%
Sign Control	Stop						
Traffic Vol by Lane	308	69	47	155	206	126	318
LT Vol	0	0	28	93	0	126	0
Through Vol	308	0	19	62	0	0	282
RT Vol	0	69	0	0	206	0	36
Lane Flow Rate	335	75	51	168	224	137	346
Geometry Grp	7	7	6	7	7	7	7
Degree of Util (X)	0.642	0.129	0.117	0.353	0.406	0.278	0.645
Departure Headway (Hd)	6.9	6.188	8.261	7.542	6.52	7.306	6.714
Convergence, Y/N	Yes						
Cap	526	580	434	478	553	493	539
Service Time	4.637	3.921	6.317	5.261	4.239	5.028	4.436
HCM Lane V/C Ratio	0.637	0.129	0.118	0.351	0.405	0.278	0.642
HCM Control Delay	21.2	9.8	12.4	14.3	13.6	12.8	20.9
HCM Lane LOS	C	A	B	B	B	B	C
HCM 95th-tile Q	4.5	0.4	0.4	1.6	2	1.1	4.6

Intersection

Intersection Delay, s/veh 11.6

Intersection LOS B

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	175	5	1	207	162	214
Future Vol, veh/h	175	5	1	207	162	214
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	5	5	4	4	3	3
Mvmt Flow	194	6	1	230	180	238
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	11.2		10.5		12.3	
HCM LOS	B		B		B	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	0%	97%	0%
Vol Thru, %	100%	0%	43%
Vol Right, %	0%	3%	57%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	208	180	376
LT Vol	1	175	0
Through Vol	207	0	162
RT Vol	0	5	214
Lane Flow Rate	231	200	418
Geometry Grp	1	1	1
Degree of Util (X)	0.321	0.311	0.518
Departure Headway (Hd)	5.004	5.591	4.467
Convergence, Y/N	Yes	Yes	Yes
Cap	712	636	802
Service Time	3.083	3.691	2.53
HCM Lane V/C Ratio	0.324	0.314	0.521
HCM Control Delay	10.5	11.2	12.3
HCM Lane LOS	B	B	B
HCM 95th-tile Q	1.4	1.3	3

Intersection

Int Delay, s/veh 5.4

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	36	241	431	61	161	221
Future Vol, veh/h	36	241	431	61	161	221
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	85	85	85	85	85	85
Heavy Vehicles, %	3	3	2	2	4	4
Mvmt Flow	42	284	507	72	189	260

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1145	507	0	0 507 0
Stage 1	507	-	-	- - -
Stage 2	638	-	-	- - -
Critical Hdwy	6.2	6.23	-	- 4.14 -
Critical Hdwy Stg 1	5.43	-	-	- - -
Critical Hdwy Stg 2	5.43	-	-	- - -
Follow-up Hdwy	2.2	2.2	-	- 2.236 -
Pot Cap-1 Maneuver	317	791	-	- 1048 -
Stage 1	886	-	-	- - -
Stage 2	755	-	-	- - -
Platoon blocked, %	-	-	-	- - -
Mov Cap-1 Maneuver	250	791	-	- 1048 -
Mov Cap-2 Maneuver	250	-	-	- - -
Stage 1	886	-	-	- - -
Stage 2	596	-	-	- - -

Approach	WB	NB	SB
HCM Control Delay, s	17.2	0	3.9
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	617	1048	-
HCM Lane V/C Ratio	-	-	0.528	0.181	-
HCM Control Delay (s)	-	-	17.2	9.2	0
HCM Lane LOS	-	-	C	A	A
HCM 95th %tile Q(veh)	-	-	3.1	0.7	-

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

04/21/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	229	562	121	134	589	105	203	564	78	181	714	213
Future Volume (vph)	229	562	121	134	589	105	203	564	78	181	714	213
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	0.99		1.00	1.00		1.00	1.00		1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.97		1.00	0.98		1.00	0.98		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3418		1770	3450		1770	3467		1787	3574	1546
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3418		1770	3450		1770	3467		1787	3574	1546
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	239	585	126	140	614	109	211	588	81	189	744	222
RTOR Reduction (vph)	0	16	0	0	12	0	0	10	0	0	0	119
Lane Group Flow (vph)	239	695	0	140	711	0	211	659	0	189	744	103
Confl. Peds. (#/hr)			16			2			3			10
Confl. Bikes (#/hr)			1			1						2
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	1%	1%	1%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	26.6	34.8		20.0	28.2		18.2	31.8		15.8	29.7	29.7
Effective Green, g (s)	26.6	34.8		20.0	28.2		18.2	31.8		15.8	29.7	29.7
Actuated g/C Ratio	0.22	0.29		0.17	0.23		0.15	0.27		0.13	0.25	0.25
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	392	991		295	810		268	918		235	884	382
v/s Ratio Prot	c0.14	0.20		0.08	c0.21		c0.12	0.19		0.11	c0.21	
v/s Ratio Perm												0.07
v/c Ratio	0.61	0.70		0.47	0.88		0.79	0.72		0.80	0.84	0.27
Uniform Delay, d1	42.0	38.0		45.2	44.2		49.0	40.0		50.6	42.9	36.4
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	6.9	4.1		5.4	12.9		14.1	2.7		17.8	7.3	0.4
Delay (s)	48.9	42.1		50.6	57.1		63.2	42.7		68.4	50.2	36.8
Level of Service	D	D		D	E		E	D		E	D	D
Approach Delay (s)		43.8			56.1			47.6			50.6	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay		49.5										D
HCM 2000 Volume to Capacity ratio		0.78										
Actuated Cycle Length (s)		120.0										17.6
Intersection Capacity Utilization		77.7%										D
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

04/21/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	229	562	121	134	589	105	203	564	78	181	714	213
Future Volume (veh/h)	229	562	121	134	589	105	203	564	78	181	714	213
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			0.99	1.00		1.00	1.00	0.97
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1885	1885	1885
Adj Flow Rate, veh/h	239	585	126	140	614	109	211	588	81	189	744	222
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	1	1	1
Cap, veh/h	312	1043	224	214	918	163	240	791	109	217	853	371
Arrive On Green	0.17	0.36	0.36	0.12	0.31	0.31	0.13	0.25	0.25	0.12	0.24	0.24
Sat Flow, veh/h	1781	2895	621	1781	3009	533	1781	3137	431	1795	3582	1556
Grp Volume(v), veh/h	239	358	353	140	362	361	211	332	337	189	744	222
Grp Sat Flow(s), veh/h/ln	1781	1777	1739	1781	1777	1765	1781	1777	1791	1795	1791	1556
Q Serve(g_s), s	15.3	19.4	19.5	9.0	21.3	21.4	13.9	20.7	20.8	12.4	24.0	15.2
Cycle Q Clear(g_c), s	15.3	19.4	19.5	9.0	21.3	21.4	13.9	20.7	20.8	12.4	24.0	15.2
Prop In Lane	1.00			1.00			0.30	1.00		0.24	1.00	1.00
Lane Grp Cap(c), veh/h	312	640	627	214	542	539	240	448	452	217	853	371
V/C Ratio(X)	0.77	0.56	0.56	0.65	0.67	0.67	0.88	0.74	0.75	0.87	0.87	0.60
Avail Cap(c_a), veh/h	312	640	627	214	542	539	310	518	522	256	940	408
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.2	30.8	30.8	50.4	36.4	36.4	50.9	41.3	41.3	51.8	43.9	40.6
Incr Delay (d2), s/veh	16.4	3.5	3.6	14.6	6.4	6.5	19.8	4.9	4.9	23.5	8.4	2.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	8.1	8.7	8.6	4.8	10.0	10.0	7.5	9.5	9.7	6.9	11.4	5.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	63.6	34.3	34.4	65.0	42.8	42.9	70.7	46.1	46.3	75.3	52.4	42.7
LnGrp LOS	E	C	C	E	D	D	E	D	D	E	D	D
Approach Vol, veh/h	950				863			880			1155	
Approach Delay, s/veh	41.7				46.4			52.1			54.3	
Approach LOS	D				D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	18.4	48.0	18.5	35.1	25.0	41.4	20.2	33.4				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	14.4	35.9	17.1	35.0	21.0	29.3	20.9	* 32				
Max Q Clear Time (g_c+l1), s	11.0	21.5	14.4	22.8	17.3	23.4	15.9	26.0				
Green Ext Time (p_c), s	0.1	3.6	0.1	3.2	0.2	2.1	0.2	2.6				
Intersection Summary												
HCM 6th Ctrl Delay				48.9								
HCM 6th LOS				D								
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection

Intersection Delay, s/veh30.7

Intersection LOS D

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	71	55	0	73	66	61	1	487	185	102	384	28
Future Vol, veh/h	71	55	0	73	66	61	1	487	185	102	384	28
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Heavy Vehicles, %	0	0	0	3	3	3	1	1	1	1	1	1
Mvmt Flow	72	56	0	74	67	62	1	492	187	103	388	28
Number of Lanes	0	1	0	0	1	1	0	1	1	1	1	0
Approach												
Opposing Approach	WB			WB			SB			NB		
Opposing Lanes	2			1			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			1			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			2			1		
HCM Control Delay	15.3			14.1			40			28.8		
HCM LOS	C			B			E			D		

Lane	NBLn1	NBLn2	EBLn1	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	0%	0%	56%	53%	0%	100%	0%
Vol Thru, %	100%	0%	44%	47%	0%	0%	93%
Vol Right, %	0%	100%	0%	0%	100%	0%	7%
Sign Control	Stop						
Traffic Vol by Lane	488	185	126	139	61	102	412
LT Vol	1	0	71	73	0	102	0
Through Vol	487	0	55	66	0	0	384
RT Vol	0	185	0	0	61	0	28
Lane Flow Rate	493	187	127	140	62	103	416
Geometry Grp	7	7	6	7	7	7	7
Degree of Util (X)	0.935	0.317	0.301	0.331	0.128	0.216	0.806
Departure Headway (Hd)	6.832	6.115	8.527	8.475	7.482	7.533	6.972
Convergence, Y/N	Yes						
Cap	532	587	420	424	478	476	520
Service Time	4.585	3.867	6.599	6.236	5.242	5.288	4.727
HCM Lane V/C Ratio	0.927	0.319	0.302	0.33	0.13	0.216	0.8
HCM Control Delay	50.7	11.7	15.3	15.4	11.3	12.4	32.8
HCM Lane LOS	F	B	C	C	B	B	D
HCM 95th-tile Q	11.6	1.4	1.2	1.4	0.4	0.8	7.7

Intersection

Intersection Delay, s/veh 17.8

Intersection LOS C

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	332	0	3	334	201	249
Future Vol, veh/h	332	0	3	334	201	249
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	1	1	2	2	2	2
Mvmt Flow	342	0	3	344	207	257
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	18		15.9		19.1	
HCM LOS	C		C		C	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	1%	100%	0%
Vol Thru, %	99%	0%	45%
Vol Right, %	0%	0%	55%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	337	332	450
LT Vol	3	332	0
Through Vol	334	0	201
RT Vol	0	0	249
Lane Flow Rate	347	342	464
Geometry Grp	1	1	1
Degree of Util (X)	0.557	0.591	0.683
Departure Headway (Hd)	5.771	6.217	5.299
Convergence, Y/N	Yes	Yes	Yes
Cap	622	578	677
Service Time	3.843	4.283	3.366
HCM Lane V/C Ratio	0.558	0.592	0.685
HCM Control Delay	15.9	18	19.1
HCM Lane LOS	C	C	C
HCM 95th-tile Q	3.4	3.8	5.4

Intersection

Int Delay, s/veh 4.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	23	280	288	173	299	310
Future Vol, veh/h	23	280	288	173	299	310
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	1	1	3	3
Mvmt Flow	24	295	303	182	315	326

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1259	303	0	0
Stage 1	303	-	-	-
Stage 2	956	-	-	-
Critical Hdwy	6.42	6.22	-	4.13
Critical Hdwy Stg 1	5.42	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-
Follow-up Hdwy	2.2	2.2	-	2.227
Pot Cap-1 Maneuver	248	1062	-	1252
Stage 1	1136	-	-	-
Stage 2	512	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	172	1062	-	1252
Mov Cap-2 Maneuver	172	-	-	-
Stage 1	1136	-	-	-
Stage 2	355	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.1	0	4.3
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	763	1252	-
HCM Lane V/C Ratio	-	-	0.418	0.251	-
HCM Control Delay (s)	-	-	13.1	8.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	2.1	1	-

Trip Generation Calculation Worksheet



Land Use Description: Single-Family Detached Housing

ITE Land Use Code: 210

Independent Variable: Dwelling Units

Quantity: 183 Dwelling Units

Summary of ITE Trip Generation Data

AM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.74 trips per dwelling unit

Directional Distribution: 25% Entering 75% Exiting

PM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.99 trips per dwelling unit

Directional Distribution: 63% Entering 37% Exiting

Total Weekday Traffic

Trip Rate: 9.44 trips per dwelling unit

Directional Distribution: 50% Entering 50% Exiting

Site Trip Generation Calculations

183 Dwelling Units

	Entering	Exiting	Total
AM Peak Hour	34	101	135
PM Peak Hour	114	67	181
Weekday	864	864	1728

Site id	HWY	MP	DIR	HS	Description	2017	2018	2019	2039	RSQ
1743	026	3.41	1		0.05 mile west of SE Foster Road		43700		44400	MODEL
1744	026	3.55	1		0.02 mile west of SE 52nd Avenue	25200			30600	MODEL
1745	026	3.59	1		0.02 mile east of SE 52nd Avenue	26200			29500	MODEL
1746	026	4.06	1		0.02 mile east of SE 62nd Avenue	27100			29900	MODEL
1747	026	4.56	1		0.02 mile east of SE 72nd Avenue	26800			31400	MODEL
1748	026	5.02	1		0.02 mile west of Cascade Highway North (OR213)	27100			32100	MODEL
1749	026	5.16	1		0.05 mile west of SE 84th Avenue	26900			29400	MODEL
1750	026	5.67	1		0.07 mile west of East Portland Freeway (I-205)	39600			43200	MODEL
1751	026	5.82	1		0.08 mile east of East Portland Freeway (I-205)	22200			23100	MODEL
1752	026	6.69	1		0.02 mile west of SE 112th Avenue	19400			20800	MODEL
1753	026	6.73	1		0.02 mile east of SE 112th Avenue	20300			21800	MODEL
1754	026	7.19	1		0.02 mile west of SE 122nd Avenue	16200			17300	MODEL
1755	026	7.23	1		0.02 mile east of SE 122nd Avenue	21500			23600	MODEL
1756	026	7.92	1		0.02 mile east of SE 136th Avenue	21500			25900	MODEL
1757	026	8.34	1		0.02 mile west of SE 144th Avenue	21200			25500	MODEL
1758	026	8.42	1		0.02 mile east of SE 145th Avenue	20200			24300	MODEL
1759	026	9.37	1		0.02 mile east of SE 164th Avenue	20800			26200	MODEL
1760	026	9.96	1		0.09 mile east of SE 174th Avenue, west city limits of Gresham	20100			20900	MODEL
					Gresham Automatic Traffic Recorder, Sta. 26-003, 0.18 mile southeast of SE Powell Valley Road				42900	MODEL
26003	026	14.36	1		0.05 mile south of SE Palmquist Road	28500			36100	MODEL
1774	026	14.80	1		0.05 mile northwest of SE Haley Road	25600			37100	MODEL
1775	026	18.30	1		0.30 mile northwest of Clackamas-Boring Highway (OR212)	25500			36900	MODEL
1776	026	19.24	1			30300			43400	MODEL
22590	026	20.60	1		0.50 mile northwest of SE Kelso Road	30300			42500	MODEL
1777	026	21.40	1		0.30 mile southeast of SE Kelso Road	33700			47300	MODEL
					0.02 mile northwest of SE 362nd Drive, west city limits of Sandy	33300			47100	MODEL
1778	026	22.72	1			15700			22400	MODEL
1779	026	23.85	1		0.02 mile west of Bluff Road					
1780	026	23.89	1		0.02 mile east of Bluff Road					

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	200	467	99	70	496	130	252	899	55	71	308	147
Future Volume (vph)	200	467	99	70	496	130	252	899	55	71	308	147
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.97		1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1719	3337		1719	3318		1770	3503		1687	3374	1436
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1719	3337		1719	3318		1770	3503		1687	3374	1436
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	217	508	108	76	539	141	274	977	60	77	335	160
RTOR Reduction (vph)	0	23	0	0	30	0	0	5	0	0	0	125
Lane Group Flow (vph)	217	593	0	76	650	0	274	1032	0	77	335	35
Confl. Peds. (#/hr)					5		5		10			28
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	2%	2%	2%	7%	7%	7%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	13.5	22.8		8.8	18.1		13.6	26.1		4.7	17.5	17.5
Effective Green, g (s)	13.5	22.8		8.8	18.1		13.6	26.1		4.7	17.5	17.5
Actuated g/C Ratio	0.17	0.29		0.11	0.23		0.17	0.33		0.06	0.22	0.22
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	290	951		189	750		300	1142		99	738	314
v/s Ratio Prot	c0.13	0.18		0.04	c0.20		c0.15	c0.29		0.05	0.10	
v/s Ratio Perm												0.02
v/c Ratio	0.75	0.62		0.40	0.87		0.91	0.90		0.78	0.45	0.11
Uniform Delay, d1	31.6	24.9		33.2	29.8		32.6	25.7		37.1	27.1	25.0
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	16.1	3.1		6.3	12.8		30.4	10.1		31.0	0.4	0.2
Delay (s)	47.8	28.0		39.4	42.6		63.0	35.8		68.1	27.6	25.2
Level of Service	D	C		D	D		E	D		E	C	C
Approach Delay (s)		33.1			42.3			41.5			32.4	
Approach LOS		C			D			D			C	
Intersection Summary												
HCM 2000 Control Delay		38.1										D
HCM 2000 Volume to Capacity ratio		0.90										
Actuated Cycle Length (s)		80.0										17.6
Intersection Capacity Utilization		74.5%										D
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/06/2021

Movement	EBL	EBT	EBC	WBL	WBT	WBC	NBL	NBT	NBC	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	200	467	99	70	496	130	252	899	55	71	308	147
Future Volume (veh/h)	200	467	99	70	496	130	252	899	55	71	308	147
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		0.99	1.00		0.99	1.00	0.96
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1826	1826	1826	1826	1826	1826	1870	1870	1870	1796	1796	1796
Adj Flow Rate, veh/h	217	508	108	76	539	141	274	977	60	77	335	160
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	5	5	5	5	5	5	2	2	2	7	7	7
Cap, veh/h	241	935	198	139	734	191	298	1069	66	97	696	298
Arrive On Green	0.14	0.33	0.33	0.08	0.27	0.27	0.17	0.31	0.31	0.06	0.20	0.20
Sat Flow, veh/h	1739	2847	602	1739	2720	709	1781	3399	209	1711	3413	1460
Grp Volume(v), veh/h	217	309	307	76	343	337	274	511	526	77	335	160
Grp Sat Flow(s), veh/h/ln	1739	1735	1714	1739	1735	1694	1781	1777	1830	1711	1706	1460
Q Serve(g_s), s	9.8	11.6	11.7	3.4	14.4	14.5	12.1	22.1	22.1	3.6	6.9	7.8
Cycle Q Clear(g_c), s	9.8	11.6	11.7	3.4	14.4	14.5	12.1	22.1	22.1	3.6	6.9	7.8
Prop In Lane	1.00			0.35	1.00		0.42	1.00		0.11	1.00	1.00
Lane Grp Cap(c), veh/h	241	570	563	139	468	457	298	559	576	97	696	298
V/C Ratio(X)	0.90	0.54	0.55	0.55	0.73	0.74	0.92	0.91	0.91	0.79	0.48	0.54
Avail Cap(c_a), veh/h	241	570	563	139	468	457	298	573	590	124	789	338
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.9	21.9	22.0	35.4	26.6	26.6	32.8	26.4	26.4	37.3	28.1	28.5
Incr Delay (d2), s/veh	36.8	3.7	3.8	14.5	9.8	10.2	31.7	19.0	18.6	22.8	0.5	1.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	6.4	4.9	4.9	1.9	6.8	6.7	7.6	11.6	11.9	2.0	2.7	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	70.7	25.6	25.8	49.9	36.3	36.8	64.5	45.4	45.0	60.0	28.6	30.0
LnGrp LOS	E	C	C	D	D	D	E	D	D	E	C	C
Approach Vol, veh/h		833			756			1311			572	
Approach Delay, s/veh		37.4			37.9			49.2			33.2	
Approach LOS		D			D			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.4	31.1	8.6	30.0	15.1	26.4	17.4	21.1				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	6.4	24.4	5.8	25.8	11.1	19.7	13.4	* 19				
Max Q Clear Time (g_c+l1), s	5.4	13.7	5.6	24.1	11.8	16.5	14.1	9.8				
Green Ext Time (p_c), s	0.0	2.6	0.0	1.0	0.0	1.2	0.0	1.7				
Intersection Summary												
HCM 6th Ctrl Delay		41.3										
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM Signalized Intersection Capacity Analysis

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	22	4	83	66	268	9	417	61	148	332	39
Future Volume (vph)	32	22	4	83	66	268	9	417	61	148	332	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)					4.5		4.5		4.5	4.5	4.5	4.5
Lane Util. Factor	1.00				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00				1.00	0.99	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr _t	0.99				1.00	0.85	1.00	0.85	1.00	1.00	0.98	
Flt Protected	0.97				0.97	1.00	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1797				1827	1576		1825	1516	1732	1798
Flt Permitted		0.79				0.79	1.00		0.99	1.00	0.43	1.00
Satd. Flow (perm)		1462				1490	1576		1808	1516	788	1798
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	35	24	4	90	72	291	10	453	66	161	361	42
RTOR Reduction (vph)	0	3	0	0	0	208	0	0	36	0	8	0
Lane Group Flow (vph)	0	60	0	0	162	83	0	463	30	161	395	0
Confl. Peds. (#/hr)					4		4		4	4		
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	4%	4%	4%	4%	4%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Actuated Green, G (s)	10.1				10.1	10.1		16.3	16.3	16.3	16.3	
Effective Green, g (s)	10.1				10.1	10.1		16.3	16.3	16.3	16.3	
Actuated g/C Ratio	0.29				0.29	0.29		0.46	0.46	0.46	0.46	
Clearance Time (s)	4.5				4.5	4.5		4.5	4.5	4.5	4.5	
Vehicle Extension (s)	3.0				3.0	3.0		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	417				425	449		832	698	362	827	
v/s Ratio Prot											0.22	
v/s Ratio Perm	0.04				c0.11	0.05		c0.26	0.02	0.20		
v/c Ratio	0.14				0.38	0.18		0.56	0.04	0.44	0.48	
Uniform Delay, d1	9.4				10.1	9.5		6.9	5.3	6.5	6.6	
Progression Factor	1.00				1.00	1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2				0.6	0.2		0.8	0.0	0.9	0.4	
Delay (s)	9.6				10.7	9.7		7.7	5.3	7.4	7.0	
Level of Service	A				B	A		A	A	A	A	
Approach Delay (s)	9.6				10.1			7.4			7.1	
Approach LOS	A				B			A			A	
Intersection Summary												
HCM 2000 Control Delay		8.2			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.49										
Actuated Cycle Length (s)		35.4			Sum of lost time (s)				9.0			
Intersection Capacity Utilization		64.1%			ICU Level of Service				C			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	32	22	4	83	66	268	9	417	61	148	332	39
Future Volume (veh/h)	32	22	4	83	66	268	9	417	61	148	332	39
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1885	1885	1885	1841	1841	1841	1841	1841	1841
Adj Flow Rate, veh/h	35	24	4	90	72	291	10	453	66	161	361	42
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	1	1	1	4	4	4	4	4	4
Cap, veh/h	296	171	21	357	242	424	109	873	745	467	776	90
Arrive On Green	0.27	0.27	0.27	0.27	0.27	0.27	0.48	0.48	0.48	0.48	0.48	0.48
Sat Flow, veh/h	519	642	79	745	908	1590	11	1821	1553	867	1618	188
Grp Volume(v), veh/h	63	0	0	162	0	291	463	0	66	161	0	403
Grp Sat Flow(s), veh/h/ln	1240	0	0	1653	0	1590	1831	0	1553	867	0	1806
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	5.8	0.0	0.0	0.8	5.6	0.0	5.3
Cycle Q Clear(g_c), s	2.5	0.0	0.0	2.5	0.0	5.8	6.2	0.0	0.8	11.8	0.0	5.3
Prop In Lane	0.56			0.56		1.00	0.02		1.00	1.00		0.10
Lane Grp Cap(c), veh/h	489	0	0	599	0	424	982	0	745	467	0	866
V/C Ratio(X)	0.13	0.00	0.00	0.27	0.00	0.69	0.47	0.00	0.09	0.34	0.00	0.47
Avail Cap(c_a), veh/h	810	0	0	999	0	828	1768	0	1421	844	0	1652
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	9.9	0.0	0.0	10.4	0.0	11.7	6.4	0.0	5.0	10.5	0.0	6.2
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.2	0.0	2.0	0.4	0.0	0.1	0.4	0.0	0.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.3	0.0	0.0	0.9	0.0	1.9	1.2	0.0	0.1	0.7	0.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	10.0	0.0	0.0	10.7	0.0	13.7	6.8	0.0	5.1	11.0	0.0	6.6
LnGrp LOS	B	A	A	B	A	B	A	A	A	B	A	A
Approach Vol, veh/h	63			453			529			564		
Approach Delay, s/veh	10.0			12.6			6.6			7.8		
Approach LOS	B			B			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+R _c), s	21.5			14.0			21.5			14.0		
Change Period (Y+R _c), s	4.5			4.5			4.5			4.5		
Max Green Setting (Gmax), s	32.5			18.5			32.5			18.5		
Max Q Clear Time (g_c+l1), s	8.2			4.5			13.8			7.8		
Green Ext Time (p_c), s	3.0			0.2			3.2			1.5		
Intersection Summary												
HCM 6th Ctrl Delay				8.8								
HCM 6th LOS				A								

Intersection

Intersection Delay, s/veh 17.5

Intersection LOS C

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	220	14	4	226	200	308
Future Vol, veh/h	220	14	4	226	200	308
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	5	5	4	4	3	3
Mvmt Flow	244	16	4	251	222	342
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	14		12.3		21.4	
HCM LOS	B		B		C	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	2%	94%	0%
Vol Thru, %	98%	0%	39%
Vol Right, %	0%	6%	61%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	230	234	508
LT Vol	4	220	0
Through Vol	226	0	200
RT Vol	0	14	308
Lane Flow Rate	256	260	564
Geometry Grp	1	1	1
Degree of Util (X)	0.396	0.444	0.757
Departure Headway (Hd)	5.574	6.142	4.825
Convergence, Y/N	Yes	Yes	Yes
Cap	644	585	751
Service Time	3.622	4.188	2.864
HCM Lane V/C Ratio	0.398	0.444	0.751
HCM Control Delay	12.3	14	21.4
HCM Lane LOS	B	B	C
HCM 95th-tile Q	1.9	2.3	7.1

Intersection

Int Delay, s/veh 9.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	57	400	450	72	208	231
Future Vol, veh/h	57	400	450	72	208	231
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	2	2	4	4
Mvmt Flow	60	421	474	76	219	243

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1155	474	0	0
Stage 1	474	-	-	-
Stage 2	681	-	-	-
Critical Hdwy	6.2	6.23	-	4.14
Critical Hdwy Stg 1	5.43	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-
Follow-up Hdwy	2.2	2.2	-	2.236
Pot Cap-1 Maneuver	312	830	-	1078
Stage 1	922	-	-	-
Stage 2	716	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	239	830	-	1078
Mov Cap-2 Maneuver	239	-	-	-
Stage 1	922	-	-	-
Stage 2	548	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	26.3	0	4.4
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	634	1078	-
HCM Lane V/C Ratio	-	-	0.759	0.203	-
HCM Control Delay (s)	-	-	26.3	9.2	0
HCM Lane LOS	-	-	D	A	A
HCM 95th %tile Q(veh)	-	-	6.9	0.8	-

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↓		↑	↑↑↓		↑	↑↑↓		↑	↑↑↓	↑
Traffic Volume (vph)	239	565	167	151	592	110	238	654	87	190	852	224
Future Volume (vph)	239	565	167	151	592	110	238	654	87	190	852	224
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	0.99		1.00	1.00		1.00	1.00		1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.97		1.00	0.98		1.00	0.98		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3384		1770	3447		1770	3469		1787	3574	1546
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3384		1770	3447		1770	3469		1787	3574	1546
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	249	589	174	157	617	115	248	681	91	198	888	233
RTOR Reduction (vph)	0	23	0	0	13	0	0	9	0	0	0	102
Lane Group Flow (vph)	249	740	0	157	719	0	248	763	0	198	888	131
Confl. Peds. (#/hr)			16			2			3			10
Confl. Bikes (#/hr)			1			1						2
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	1%	1%	1%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	22.1	34.6		16.8	29.3		19.2	34.6		16.4	32.1	32.1
Effective Green, g (s)	22.1	34.6		16.8	29.3		19.2	34.6		16.4	32.1	32.1
Actuated g/C Ratio	0.18	0.29		0.14	0.24		0.16	0.29		0.14	0.27	0.27
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	325	975		247	841		283	1000		244	956	413
v/s Ratio Prot	c0.14	0.22		0.09	c0.21		c0.14	c0.22		0.11	c0.25	
v/s Ratio Perm												0.08
v/c Ratio	0.77	0.76		0.64	0.86		0.88	0.76		0.81	0.93	0.32
Uniform Delay, d1	46.5	38.9		48.7	43.3		49.2	39.0		50.3	42.8	35.2
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	15.8	5.5		11.9	10.8		24.7	3.5		18.2	14.7	0.4
Delay (s)	62.3	44.4		60.6	54.2		74.0	42.5		68.5	57.5	35.6
Level of Service	E	D		E	D		E	D		E	E	D
Approach Delay (s)		48.8			55.3			50.1			55.3	
Approach LOS		D			E			D			E	
Intersection Summary												
HCM 2000 Control Delay		52.5										D
HCM 2000 Volume to Capacity ratio		0.86										
Actuated Cycle Length (s)		120.0										17.6
Intersection Capacity Utilization		84.3%										E
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/06/2021

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	239	565	167	151	592	110	238	654	87	190	852	224
Future Volume (veh/h)	239	565	167	151	592	110	238	654	87	190	852	224
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			0.99	1.00		1.00	1.00	0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1885	1885	1885
Adj Flow Rate, veh/h	249	589	174	157	617	115	248	681	91	198	888	233
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	1	1	1
Cap, veh/h	297	846	249	218	807	150	275	914	122	226	937	408
Arrive On Green	0.17	0.31	0.31	0.12	0.27	0.27	0.15	0.29	0.29	0.13	0.26	0.26
Sat Flow, veh/h	1781	2687	791	1781	2983	555	1781	3150	420	1795	3582	1558
Grp Volume(v), veh/h	249	389	374	157	367	365	248	384	388	198	888	233
Grp Sat Flow(s), veh/h/ln	1781	1777	1701	1781	1777	1761	1781	1777	1793	1795	1791	1558
Q Serve(g_s), s	16.3	23.0	23.2	10.2	22.8	22.9	16.4	23.5	23.5	13.0	29.2	15.6
Cycle Q Clear(g_c), s	16.3	23.0	23.2	10.2	22.8	22.9	16.4	23.5	23.5	13.0	29.2	15.6
Prop In Lane	1.00			1.00			0.32	1.00		0.23	1.00	1.00
Lane Grp Cap(c), veh/h	297	559	536	218	481	476	275	515	520	226	937	408
V/C Ratio(X)	0.84	0.70	0.70	0.72	0.76	0.77	0.90	0.74	0.75	0.88	0.95	0.57
Avail Cap(c_a), veh/h	297	559	536	218	481	476	297	515	520	266	940	409
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	48.4	36.1	36.1	50.7	40.2	40.3	49.8	38.6	38.6	51.5	43.5	38.5
Incr Delay (d2), s/veh	23.8	7.0	7.4	18.5	11.0	11.2	27.5	5.8	5.8	23.5	18.0	1.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	9.0	10.8	10.4	5.6	11.2	11.1	9.3	10.9	11.0	7.2	14.9	6.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	72.2	43.1	43.5	69.1	51.2	51.5	77.3	44.4	44.4	75.0	61.4	40.4
LnGrp LOS	E	D	D	E	D	D	E	D	D	E	E	D
Approach Vol, veh/h	1012				889			1020			1319	
Approach Delay, s/veh	50.4				54.5			52.4			59.8	
Approach LOS	D				D			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	18.7	42.6	19.1	39.6	24.0	37.3	22.5	36.2				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	14.7	36.5	17.8	33.4	20.0	31.2	20.0	* 32				
Max Q Clear Time (g_c+l1), s	12.2	25.2	15.0	25.5	18.3	24.9	18.4	31.2				
Green Ext Time (p_c), s	0.1	3.5	0.1	2.9	0.1	2.3	0.1	0.2				
Intersection Summary												
HCM 6th Ctrl Delay		54.6										
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM Signalized Intersection Capacity Analysis

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	57	10	71	71	97	7	577	152	162	510	32
Future Volume (vph)	75	57	10	71	71	97	7	577	152	162	510	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)					4.5		4.5		4.5	4.5	4.5	4.5
Lane Util. Factor		1.00				1.00	1.00		1.00	1.00	1.00	1.00
Frt		0.99				1.00	0.85		1.00	0.85	1.00	0.99
Flt Protected		0.97				0.98	1.00		1.00	1.00	0.95	1.00
Satd. Flow (prot)				1834			1800	1568		1880	1599	1787
Flt Permitted				0.76			0.83	1.00		0.99	1.00	0.37
Satd. Flow (perm)				1428			1529	1568		1870	1599	689
Peak-hour factor, PHF	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Adj. Flow (vph)	76	58	10	72	72	98	7	583	154	164	515	32
RTOR Reduction (vph)	0	5	0	0	0	78	0	0	67	0	3	0
Lane Group Flow (vph)	0	139	0	0	144	20	0	590	87	164	544	0
Heavy Vehicles (%)	0%	0%	0%	3%	3%	3%	1%	1%	1%	1%	1%	1%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4				8			2			6
Permitted Phases	4				8		8	2		2		6
Actuated Green, G (s)		8.0				8.0	8.0		22.2	22.2	22.2	22.2
Effective Green, g (s)		8.0				8.0	8.0		22.2	22.2	22.2	22.2
Actuated g/C Ratio		0.20				0.20	0.20		0.57	0.57	0.57	0.57
Clearance Time (s)		4.5				4.5	4.5		4.5	4.5	4.5	4.5
Vehicle Extension (s)		3.0				3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		291				312	320		1059	905	390	1056
v/s Ratio Prot												0.29
v/s Ratio Perm		c0.10				0.09	0.01		c0.32	0.05	0.24	
v/c Ratio		0.48				0.46	0.06		0.56	0.10	0.42	0.51
Uniform Delay, d1		13.8				13.7	12.6		5.4	3.9	4.8	5.2
Progression Factor		1.00				1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2		1.2				1.1	0.1		0.6	0.0	0.7	0.4
Delay (s)		15.0				14.8	12.7		6.0	3.9	5.6	5.6
Level of Service		B				B	B		A	A	A	A
Approach Delay (s)		15.0				13.9			5.6			5.6
Approach LOS		B				B			A			A
Intersection Summary												
HCM 2000 Control Delay		7.4					HCM 2000 Level of Service			A		
HCM 2000 Volume to Capacity ratio		0.54										
Actuated Cycle Length (s)		39.2					Sum of lost time (s)			9.0		
Intersection Capacity Utilization		85.2%					ICU Level of Service			E		
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/06/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	75	57	10	71	71	97	7	577	152	162	510	32
Future Volume (veh/h)	75	57	10	71	71	97	7	577	152	162	510	32
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1856	1856	1856	1885	1885	1885	1885	1885	1885
Adj Flow Rate, veh/h	76	58	10	72	72	98	7	583	154	164	515	32
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Percent Heavy Veh, %	0	0	0	3	3	3	1	1	1	1	1	1
Cap, veh/h	243	127	16	277	192	290	105	1055	899	466	988	61
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.56	0.56	0.56	0.56	0.56	0.56
Sat Flow, veh/h	482	687	87	680	1044	1572	5	1875	1598	727	1756	109
Grp Volume(v), veh/h	144	0	0	144	0	98	590	0	154	164	0	547
Grp Sat Flow(s), veh/h/ln	1256	0	0	1724	0	1572	1879	0	1598	727	0	1866
Q Serve(g_s), s	1.7	0.0	0.0	0.0	0.0	1.9	0.0	0.0	1.7	6.6	0.0	6.5
Cycle Q Clear(g_c), s	4.2	0.0	0.0	2.5	0.0	1.9	7.1	0.0	1.7	13.7	0.0	6.5
Prop In Lane	0.53			0.50			1.00	0.01		1.00	1.00	0.06
Lane Grp Cap(c), veh/h	386	0	0	470	0	290	1160	0	899	466	0	1050
V/C Ratio(X)	0.37	0.00	0.00	0.31	0.00	0.34	0.51	0.00	0.17	0.35	0.00	0.52
Avail Cap(c_a), veh/h	857	0	0	966	0	795	1838	0	1481	731	0	1730
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	13.5	0.0	0.0	12.8	0.0	12.6	5.0	0.0	3.8	9.3	0.0	4.8
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.4	0.0	0.7	0.3	0.0	0.1	0.5	0.0	0.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.9	0.0	0.0	0.9	0.0	0.6	1.0	0.0	0.2	0.7	0.0	0.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	14.1	0.0	0.0	13.2	0.0	13.3	5.3	0.0	3.9	9.8	0.0	5.2
LnGrp LOS	B	A	A	B	A	B	A	A	A	A	A	A
Approach Vol, veh/h	144				242			744			711	
Approach Delay, s/veh	14.1				13.2			5.0			6.3	
Approach LOS	B				B			A			A	
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+R _c), s	24.5			11.1			24.5			11.1		
Change Period (Y+R _c), s	4.5			4.5			4.5			4.5		
Max Green Setting (Gmax), s	33.0			18.0			33.0			18.0		
Max Q Clear Time (g_c+l1), s	9.1			6.2			15.7			4.5		
Green Ext Time (p_c), s	4.3			0.5			4.3			0.9		
Intersection Summary												
HCM 6th Ctrl Delay				7.3								
HCM 6th LOS				A								

Intersection

Intersection Delay, s/veh 46.6

Intersection LOS E

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	446	5	12	383	231	323
Future Vol, veh/h	446	5	12	383	231	323
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	1	1	2	2	2	2
Mvmt Flow	460	5	12	395	238	333
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	45.4		29.7		59.6	
HCM LOS	E		D		F	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	3%	99%	0%
Vol Thru, %	97%	0%	42%
Vol Right, %	0%	1%	58%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	395	451	554
LT Vol	12	446	0
Through Vol	383	0	231
RT Vol	0	5	323
Lane Flow Rate	407	465	571
Geometry Grp	1	1	1
Degree of Util (X)	0.774	0.9	0.988
Departure Headway (Hd)	6.843	6.967	6.226
Convergence, Y/N	Yes	Yes	Yes
Cap	525	521	583
Service Time	4.918	5.032	4.292
HCM Lane V/C Ratio	0.775	0.893	0.979
HCM Control Delay	29.7	45.4	59.6
HCM Lane LOS	D	E	F
HCM 95th-tile Q	7	10.3	14.1

Intersection

Int Delay, s/veh 14.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	37	376	302	203	452	324
Future Vol, veh/h	37	376	302	203	452	324
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	1	1	3	3
Mvmt Flow	39	396	318	214	476	341

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1611	318	0	0	318
Stage 1	318	-	-	-	-
Stage 2	1293	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.13
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	2.2	2.2	-	-	2.227
Pot Cap-1 Maneuver	145	1039	-	-	1236
Stage 1	1116	-	-	-	-
Stage 2	338	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	76	1039	-	-	1236
Mov Cap-2 Maneuver	76	-	-	-	-
Stage 1	1116	-	-	-	-
Stage 2	177	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	47.8	0	5.7
HCM LOS	E		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	487	1236	-
HCM Lane V/C Ratio	-	-	0.893	0.385	-
HCM Control Delay (s)	-	-	47.8	9.7	0
HCM Lane LOS	-	-	E	A	A
HCM 95th %tile Q(veh)	-	-	9.9	1.8	-

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	200	467	102	70	496	130	262	924	55	71	317	147
Future Volume (vph)	200	467	102	70	496	130	262	924	55	71	317	147
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.97		1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1719	3334		1719	3318		1770	3504		1687	3374	1436
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1719	3334		1719	3318		1770	3504		1687	3374	1436
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	217	508	111	76	539	141	285	1004	60	77	345	160
RTOR Reduction (vph)	0	24	0	0	30	0	0	5	0	0	0	125
Lane Group Flow (vph)	217	595	0	76	650	0	285	1059	0	77	345	35
Confl. Peds. (#/hr)					5		5		10			28
Heavy Vehicles (%)	5%	5%	5%	5%	5%	5%	2%	2%	2%	7%	7%	7%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	13.4	22.4		8.7	17.7		14.1	26.7		4.6	17.5	17.5
Effective Green, g (s)	13.4	22.4		8.7	17.7		14.1	26.7		4.6	17.5	17.5
Actuated g/C Ratio	0.17	0.28		0.11	0.22		0.18	0.33		0.06	0.22	0.22
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	287	933		186	734		311	1169		97	738	314
v/s Ratio Prot	c0.13	0.18		0.04	c0.20		c0.16	c0.30		0.05	0.10	
v/s Ratio Perm												0.02
v/c Ratio	0.76	0.64		0.41	0.89		0.92	0.91		0.79	0.47	0.11
Uniform Delay, d1	31.7	25.2		33.3	30.2		32.4	25.4		37.2	27.2	25.0
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	16.9	3.3		6.5	14.8		30.1	10.0		34.6	0.5	0.2
Delay (s)	48.6	28.6		39.8	45.0		62.5	35.5		71.9	27.7	25.2
Level of Service	D	C		D	D		E	D		E	C	C
Approach Delay (s)		33.8			44.5			41.2			32.8	
Approach LOS		C			D			D			C	
Intersection Summary												
HCM 2000 Control Delay		38.8										D
HCM 2000 Volume to Capacity ratio		0.91										
Actuated Cycle Length (s)		80.0										17.6
Intersection Capacity Utilization		75.2%										D
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/07/2021

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	200	467	102	70	496	130	262	924	55	71	317	147
Future Volume (veh/h)	200	467	102	70	496	130	262	924	55	71	317	147
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.99	1.00		0.99	1.00		0.96
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No		No		No		No	No		No
Adj Sat Flow, veh/h/ln	1826	1826	1826	1826	1826	1826	1870	1870	1870	1796	1796	1796
Adj Flow Rate, veh/h	217	508	111	76	539	141	285	1004	60	77	345	160
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	5	5	5	5	5	5	2	2	2	7	7	7
Cap, veh/h	241	909	198	139	713	186	312	1097	66	97	696	298
Arrive On Green	0.14	0.32	0.32	0.08	0.26	0.26	0.17	0.32	0.32	0.06	0.20	0.20
Sat Flow, veh/h	1739	2831	615	1739	2720	708	1781	3405	203	1711	3413	1460
Grp Volume(v), veh/h	217	310	309	76	343	337	285	524	540	77	345	160
Grp Sat Flow(s), veh/h/ln	1739	1735	1712	1739	1735	1694	1781	1777	1831	1711	1706	1460
Q Serve(g_s), s	9.8	11.8	12.0	3.4	14.5	14.7	12.6	22.7	22.7	3.6	7.2	7.8
Cycle Q Clear(g_c), s	9.8	11.8	12.0	3.4	14.5	14.7	12.6	22.7	22.7	3.6	7.2	7.8
Prop In Lane	1.00		0.36	1.00		0.42	1.00		0.11	1.00		1.00
Lane Grp Cap(c), veh/h	241	557	549	139	455	444	312	572	590	97	696	298
V/C Ratio(X)	0.90	0.56	0.56	0.55	0.75	0.76	0.91	0.92	0.92	0.79	0.50	0.54
Avail Cap(c_a), veh/h	241	557	549	139	455	444	312	586	604	124	789	338
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.9	22.5	22.5	35.4	27.1	27.2	32.4	26.1	26.1	37.3	28.2	28.5
Incr Delay (d2), s/veh	36.8	4.0	4.1	14.5	11.0	11.5	30.0	18.9	18.5	22.8	0.5	1.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	6.4	5.1	5.1	1.9	7.0	6.9	7.8	11.9	12.2	2.0	2.8	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	70.7	26.5	26.6	49.9	38.2	38.7	62.4	45.0	44.6	60.0	28.7	30.0
LnGrp LOS	E	C	C	D	D	D	E	D	D	E	C	C
Approach Vol, veh/h		836			756			1349			582	
Approach Delay, s/veh		38.0			39.6			48.5			33.2	
Approach LOS		D			D			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.4	30.5	8.6	30.6	15.1	25.8	18.0	21.1				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	6.4	23.8	5.8	26.4	11.1	19.1	14.0	* 19				
Max Q Clear Time (g_c+l1), s	5.4	14.0	5.6	24.7	11.8	16.7	14.6	9.8				
Green Ext Time (p_c), s	0.0	2.5	0.0	1.1	0.0	1.0	0.0	1.8				
Intersection Summary												
HCM 6th Ctrl Delay		41.6										
HCM 6th LOS			D									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM Signalized Intersection Capacity Analysis

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	22	4	85	66	268	9	457	66	148	346	39
Future Volume (vph)	32	22	4	85	66	268	9	457	66	148	346	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)					4.5		4.5		4.5	4.5	4.5	4.5
Lane Util. Factor	1.00				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00				1.00	0.99	1.00	0.98	1.00	1.00	1.00	
Flpb, ped/bikes	1.00				1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fr _t	0.99				1.00	0.85	1.00	0.85	1.00	0.99		
Flt Protected	0.97				0.97	1.00	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1797				1826	1576	1825	1516	1732	1799	
Flt Permitted		0.79				0.79	1.00	0.99	1.00	0.39	1.00	
Satd. Flow (perm)		1463				1486	1576	1809	1516	716	1799	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	35	24	4	92	72	291	10	497	72	161	376	42
RTOR Reduction (vph)	0	3	0	0	0	209	0	0	38	0	8	0
Lane Group Flow (vph)	0	60	0	0	164	82	0	507	34	161	410	0
Confl. Peds. (#/hr)					4		4		4	4		
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	4%	4%	4%	4%	4%	4%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Actuated Green, G (s)	10.4				10.4	10.4		17.5	17.5	17.5	17.5	
Effective Green, g (s)	10.4				10.4	10.4		17.5	17.5	17.5	17.5	
Actuated g/C Ratio	0.28				0.28	0.28		0.47	0.47	0.47	0.47	
Clearance Time (s)	4.5				4.5	4.5		4.5	4.5	4.5	4.5	
Vehicle Extension (s)	3.0				3.0	3.0		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	412				418	444		857	718	339	853	
v/s Ratio Prot											0.23	
v/s Ratio Perm	0.04				c0.11	0.05		c0.28	0.02	0.23		
v/c Ratio	0.15				0.39	0.18		0.59	0.05	0.47	0.48	
Uniform Delay, d1	9.9				10.7	10.0		7.1	5.2	6.6	6.6	
Progression Factor	1.00				1.00	1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2				0.6	0.2		1.1	0.0	1.1	0.4	
Delay (s)	10.1				11.3	10.2		8.2	5.2	7.6	7.0	
Level of Service	B				B	B		A	A	A	A	
Approach Delay (s)	10.1				10.6			7.8			7.2	
Approach LOS	B				B			A			A	
Intersection Summary												
HCM 2000 Control Delay		8.5			HCM 2000 Level of Service				A			
HCM 2000 Volume to Capacity ratio		0.52										
Actuated Cycle Length (s)		36.9			Sum of lost time (s)				9.0			
Intersection Capacity Utilization		67.1%			ICU Level of Service				C			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	32	22	4	85	66	268	9	457	66	148	346	39
Future Volume (veh/h)	32	22	4	85	66	268	9	457	66	148	346	39
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1885	1885	1885	1841	1841	1841	1841	1841	1841
Adj Flow Rate, veh/h	35	24	4	92	72	291	10	497	72	161	376	42
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	1	1	1	4	4	4	4	4	4
Cap, veh/h	284	165	20	350	234	417	103	907	773	446	809	90
Arrive On Green	0.26	0.26	0.26	0.26	0.26	0.26	0.50	0.50	0.50	0.50	0.50	0.50
Sat Flow, veh/h	512	628	77	762	891	1590	10	1822	1554	828	1626	182
Grp Volume(v), veh/h	63	0	0	164	0	291	507	0	72	161	0	418
Grp Sat Flow(s), veh/h/ln	1217	0	0	1654	0	1590	1832	0	1554	828	0	1807
Q Serve(g_s), s	0.1	0.0	0.0	0.0	0.0	6.2	0.0	0.0	0.9	6.3	0.0	5.7
Cycle Q Clear(g_c), s	2.7	0.0	0.0	2.7	0.0	6.2	7.2	0.0	0.9	13.4	0.0	5.7
Prop In Lane	0.56			0.56			1.00	0.02		1.00	1.00	0.10
Lane Grp Cap(c), veh/h	469	0	0	584	0	417	1009	0	773	446	0	899
V/C Ratio(X)	0.13	0.00	0.00	0.28	0.00	0.70	0.50	0.00	0.09	0.36	0.00	0.46
Avail Cap(c_a), veh/h	761	0	0	947	0	785	1676	0	1347	752	0	1566
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	10.6	0.0	0.0	11.2	0.0	12.5	6.5	0.0	5.0	11.2	0.0	6.2
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.3	0.0	2.1	0.4	0.0	0.1	0.5	0.0	0.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.3	0.0	0.0	0.9	0.0	2.0	1.4	0.0	0.2	0.8	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	10.7	0.0	0.0	11.4	0.0	14.6	6.9	0.0	5.0	11.7	0.0	6.5
LnGrp LOS	B	A	A	B	A	B	A	A	A	B	A	A
Approach Vol, veh/h	63			455			579		579			
Approach Delay, s/veh	10.7			13.5			6.7		8.0			
Approach LOS	B			B			A		A			
Timer - Assigned Phs	2			4			6		8			
Phs Duration (G+Y+R _c), s	23.2			14.3			23.2		14.3			
Change Period (Y+R _c), s	4.5			4.5			4.5		4.5			
Max Green Setting (Gmax), s	32.5			18.5			32.5		18.5			
Max Q Clear Time (g _{c+l1}), s	9.2			4.7			15.4		8.2			
Green Ext Time (p _c), s	3.3			0.2			3.2		1.5			
Intersection Summary												
HCM 6th Ctrl Delay				9.1								
HCM 6th LOS				A								

Intersection

Intersection Delay, s/veh 20.6

Intersection LOS C

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	220	21	24	276	217	308
Future Vol, veh/h	220	21	24	276	217	308
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	5	5	4	4	3	3
Mvmt Flow	244	23	27	307	241	342
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	15.1		15		26.3	
HCM LOS	C		B		D	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	8%	91%	0%
Vol Thru, %	92%	0%	41%
Vol Right, %	0%	9%	59%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	300	241	525
LT Vol	24	220	0
Through Vol	276	0	217
RT Vol	0	21	308
Lane Flow Rate	333	268	583
Geometry Grp	1	1	1
Degree of Util (X)	0.528	0.475	0.814
Departure Headway (Hd)	5.704	6.386	5.025
Convergence, Y/N	Yes	Yes	Yes
Cap	630	562	718
Service Time	3.765	4.447	3.078
HCM Lane V/C Ratio	0.529	0.477	0.812
HCM Control Delay	15	15.1	26.3
HCM Lane LOS	B	C	D
HCM 95th-tile Q	3.1	2.5	8.6

Intersection

Int Delay, s/veh 10.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	57	420	450	72	215	231
Future Vol, veh/h	57	420	450	72	215	231
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	2	2	4	4
Mvmt Flow	60	442	474	76	226	243

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1169	474	0	0
Stage 1	474	-	-	-
Stage 2	695	-	-	-
Critical Hdwy	6.2	6.23	-	4.14
Critical Hdwy Stg 1	5.43	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-
Follow-up Hdwy	2.2	2.2	-	2.236
Pot Cap-1 Maneuver	306	830	-	1078
Stage 1	922	-	-	-
Stage 2	704	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	232	830	-	1078
Mov Cap-2 Maneuver	232	-	-	-
Stage 1	922	-	-	-
Stage 2	534	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	28.7	0	4.4
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	635	1078	-
HCM Lane V/C Ratio	-	0.791	0.21	-
HCM Control Delay (s)	-	28.7	9.2	0
HCM Lane LOS	-	D	A	A
HCM 95th %tile Q(veh)	-	7.7	0.8	-

Intersection

Int Delay, s/veh 1.3

Movement WBL WBR NBT NBR SBL SBT

Lane Configurations						
Traffic Vol, veh/h	10	47	253	3	16	222
Future Vol, veh/h	10	47	253	3	16	222
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	4	2	2	3
Mvmt Flow	11	52	281	3	18	247

Major/Minor Minor1 Major1 Major2

Conflicting Flow All	566	283	0	0	284	0
Stage 1	283	-	-	-	-	-
Stage 2	283	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	486	756	-	-	1278	-
Stage 1	765	-	-	-	-	-
Stage 2	765	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	478	756	-	-	1278	-
Mov Cap-2 Maneuver	478	-	-	-	-	-
Stage 1	765	-	-	-	-	-
Stage 2	753	-	-	-	-	-

Approach WB NB SB

HCM Control Delay, s 10.8 0 0.5

HCM LOS B

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	686	1278	-
HCM Lane V/C Ratio	-	-	0.092	0.014	-
HCM Control Delay (s)	-	-	10.8	7.9	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0	-

Intersection

Int Delay, s/veh 1.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B			A	
Traffic Vol, veh/h	21	23	233	7	8	224
Future Vol, veh/h	21	23	233	7	8	224
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	4	2	2	3
Mvmt Flow	23	26	259	8	9	249

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	530	263	0	0	267
Stage 1	263	-	-	-	-
Stage 2	267	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	510	776	-	-	1297
Stage 1	781	-	-	-	-
Stage 2	778	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	506	776	-	-	1297
Mov Cap-2 Maneuver	506	-	-	-	-
Stage 1	781	-	-	-	-
Stage 2	772	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.3	0	0.3
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	618	1297	-
HCM Lane V/C Ratio	-	-	0.079	0.007	-
HCM Control Delay (s)	-	-	11.3	7.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0	-

HCM Signalized Intersection Capacity Analysis
1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	239	565	178	151	592	110	245	671	87	190	881	224
Future Volume (vph)	239	565	178	151	592	110	245	671	87	190	881	224
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95		1.00	0.95	1.00
Frpb, ped/bikes	1.00	0.99		1.00	1.00		1.00	1.00		1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Fr _t	1.00	0.96		1.00	0.98		1.00	0.98		1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3376		1770	3447		1770	3471		1787	3574	1546
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3376		1770	3447		1770	3471		1787	3574	1546
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	249	589	185	157	617	115	255	699	91	198	918	233
RTOR Reduction (vph)	0	26	0	0	13	0	0	8	0	0	0	98
Lane Group Flow (vph)	249	748	0	157	719	0	255	782	0	198	918	135
Confl. Peds. (#/hr)			16			2			3			10
Confl. Bikes (#/hr)			1			1						2
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	1%	1%	1%
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		7	4		3	8	
Permitted Phases												8
Actuated Green, G (s)	21.4	34.6		16.1	29.3		19.5	35.3		16.4	32.5	32.5
Effective Green, g (s)	21.4	34.6		16.1	29.3		19.5	35.3		16.4	32.5	32.5
Actuated g/C Ratio	0.18	0.29		0.13	0.24		0.16	0.29		0.14	0.27	0.27
Clearance Time (s)	4.0	4.8		4.0	4.8		4.0	4.8		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	315	973		237	841		287	1021		244	967	418
v/s Ratio Prot	c0.14	0.22		0.09	c0.21		c0.14	c0.23		0.11	c0.26	
v/s Ratio Perm												0.09
v/c Ratio	0.79	0.77		0.66	0.86		0.89	0.77		0.81	0.95	0.32
Uniform Delay, d1	47.2	39.0		49.4	43.3		49.2	38.6		50.3	42.9	35.0
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	18.1	5.8		13.7	10.8		26.4	3.5		18.2	17.7	0.5
Delay (s)	65.2	44.9		63.0	54.2		75.6	42.1		68.5	60.7	35.4
Level of Service	E	D		E	D		E	D		E	E	D
Approach Delay (s)		49.8			55.7			50.2			57.5	
Approach LOS		D			E			D			E	
Intersection Summary												
HCM 2000 Control Delay		53.5										D
HCM 2000 Volume to Capacity ratio		0.87										
Actuated Cycle Length (s)		120.0										17.6
Intersection Capacity Utilization		85.5%										E
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑		↑	↑↑		↑	↑↑	↑
Traffic Volume (veh/h)	239	565	178	151	592	110	245	671	87	190	881	224
Future Volume (veh/h)	239	565	178	151	592	110	245	671	87	190	881	224
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.97	1.00		0.98	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1885	1885	1885
Adj Flow Rate, veh/h	249	589	185	157	617	115	255	699	91	198	918	233
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	1	1	1
Cap, veh/h	297	820	257	218	794	148	282	931	121	226	940	409
Arrive On Green	0.17	0.31	0.31	0.12	0.27	0.27	0.16	0.29	0.29	0.13	0.26	0.26
Sat Flow, veh/h	1781	2642	828	1781	2983	555	1781	3161	411	1795	3582	1558
Grp Volume(v), veh/h	249	396	378	157	367	365	255	393	397	198	918	233
Grp Sat Flow(s), veh/h/ln	1781	1777	1693	1781	1777	1761	1781	1777	1795	1795	1791	1558
Q Serve(g_s), s	16.3	23.7	23.8	10.2	22.9	23.0	16.9	24.0	24.1	13.0	30.5	15.6
Cycle Q Clear(g_c), s	16.3	23.7	23.8	10.2	22.9	23.0	16.9	24.0	24.1	13.0	30.5	15.6
Prop In Lane	1.00		0.49	1.00		0.32	1.00		0.23	1.00		1.00
Lane Grp Cap(c), veh/h	297	551	525	218	473	469	282	523	529	226	940	409
V/C Ratio(X)	0.84	0.72	0.72	0.72	0.78	0.78	0.91	0.75	0.75	0.88	0.98	0.57
Avail Cap(c_a), veh/h	297	551	525	218	473	469	297	523	529	266	940	409
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	48.4	36.7	36.8	50.7	40.7	40.8	49.6	38.3	38.3	51.5	43.9	38.4
Incr Delay (d2), s/veh	23.8	7.8	8.3	18.5	11.8	12.1	28.6	6.0	6.0	23.5	23.6	1.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	9.0	11.2	10.8	5.6	11.3	11.3	9.7	11.2	11.3	7.2	16.2	6.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	72.2	44.5	45.0	69.1	52.5	52.8	78.2	44.3	44.3	75.0	67.5	40.3
LnGrp LOS	E	D	D	E	D	D	E	D	D	E	E	D
Approach Vol, veh/h	1023				889			1045			1349	
Approach Delay, s/veh	51.4				55.6			52.6			63.9	
Approach LOS		D				E			D		E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	18.7	42.0	19.1	40.1	24.0	36.7	23.0	36.3				
Change Period (Y+Rc), s	4.0	4.8	4.0	4.8	4.0	4.8	4.0	* 4.8				
Max Green Setting (Gmax), s	14.7	36.5	17.8	33.4	20.0	31.2	20.0	* 32				
Max Q Clear Time (g_c+l1), s	12.2	25.8	15.0	26.1	18.3	25.0	18.9	32.5				
Green Ext Time (p_c), s	0.1	3.4	0.1	2.8	0.1	2.2	0.1	0.0				
Intersection Summary												
HCM 6th Ctrl Delay				56.5								
HCM 6th LOS				E								
Notes												
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.												

HCM Signalized Intersection Capacity Analysis

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/07/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	57	10	77	71	97	7	604	156	162	556	32
Future Volume (vph)	75	57	10	77	71	97	7	604	156	162	556	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)					4.5		4.5		4.5	4.5	4.5	4.5
Lane Util. Factor		1.00				1.00	1.00		1.00	1.00	1.00	1.00
Frt		0.99				1.00	0.85		1.00	0.85	1.00	0.99
Flt Protected		0.97				0.97	1.00		1.00	1.00	0.95	1.00
Satd. Flow (prot)				1834			1798	1568		1880	1599	1787
Flt Permitted				0.76			0.82	1.00		0.99	1.00	0.35
Satd. Flow (perm)				1423			1519	1568		1870	1599	651
Peak-hour factor, PHF	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Adj. Flow (vph)	76	58	10	78	72	98	7	610	158	164	562	32
RTOR Reduction (vph)	0	5	0	0	0	78	0	0	68	0	3	0
Lane Group Flow (vph)	0	139	0	0	150	20	0	617	90	164	591	0
Heavy Vehicles (%)	0%	0%	0%	3%	3%	3%	1%	1%	1%	1%	1%	1%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4				8			2			6
Permitted Phases	4				8		8	2		2		6
Actuated Green, G (s)		8.2			8.2	8.2		22.7	22.7	22.7	22.7	
Effective Green, g (s)		8.2			8.2	8.2		22.7	22.7	22.7	22.7	
Actuated g/C Ratio		0.21				0.21	0.21		0.57	0.57	0.57	0.57
Clearance Time (s)		4.5				4.5	4.5		4.5	4.5	4.5	4.5
Vehicle Extension (s)		3.0				3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		292			312	322		1063	909	370	1061	
v/s Ratio Prot												0.32
v/s Ratio Perm		0.10			c0.10	0.01		c0.33	0.06	0.25		
v/c Ratio		0.48			0.48	0.06		0.58	0.10	0.44	0.56	
Uniform Delay, d1		14.0			14.0	12.8		5.5	3.9	5.0	5.4	
Progression Factor		1.00			1.00	1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2		1.2			1.2	0.1		0.8	0.0	0.8	0.6	
Delay (s)		15.2			15.1	12.8		6.3	4.0	5.8	6.1	
Level of Service		B			B	B		A	A	A	A	
Approach Delay (s)		15.2			14.2			5.9			6.0	
Approach LOS		B			B			A			A	
Intersection Summary												
HCM 2000 Control Delay		7.7			HCM 2000 Level of Service			A				
HCM 2000 Volume to Capacity ratio		0.55										
Actuated Cycle Length (s)		39.9			Sum of lost time (s)			9.0				
Intersection Capacity Utilization		89.1%			ICU Level of Service			E				
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary

2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

05/07/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	75	57	10	77	71	97	7	604	156	162	556	32
Future Volume (veh/h)	75	57	10	77	71	97	7	604	156	162	556	32
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1856	1856	1856	1885	1885	1885	1885	1885	1885
Adj Flow Rate, veh/h	76	58	10	78	72	98	7	610	158	164	562	32
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Percent Heavy Veh, %	0	0	0	3	3	3	1	1	1	1	1	1
Cap, veh/h	234	126	16	280	184	290	100	1077	918	452	1015	58
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.57	0.57	0.57	0.57	0.57	0.57
Sat Flow, veh/h	468	683	86	723	999	1572	5	1875	1598	706	1766	101
Grp Volume(v), veh/h	144	0	0	150	0	98	617	0	158	164	0	594
Grp Sat Flow(s), veh/h/ln	1237	0	0	1722	0	1572	1879	0	1598	706	0	1867
Q Serve(g_s), s	1.8	0.0	0.0	0.0	0.0	2.0	0.0	0.0	1.7	7.1	0.0	7.4
Cycle Q Clear(g_c), s	4.5	0.0	0.0	2.7	0.0	2.0	7.7	0.0	1.7	14.9	0.0	7.4
Prop In Lane	0.53		0.07	0.52		1.00	0.01		1.00	1.00		0.05
Lane Grp Cap(c), veh/h	375	0	0	464	0	290	1177	0	918	452	0	1073
V/C Ratio(X)	0.38	0.00	0.00	0.32	0.00	0.34	0.52	0.00	0.17	0.36	0.00	0.55
Avail Cap(c_a), veh/h	811	0	0	920	0	758	1751	0	1411	670	0	1649
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	14.2	0.0	0.0	13.5	0.0	13.2	5.0	0.0	3.8	9.7	0.0	5.0
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.4	0.0	0.7	0.4	0.0	0.1	0.5	0.0	0.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	0.9	0.0	0.0	1.0	0.0	0.7	1.1	0.0	0.2	0.7	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	14.8	0.0	0.0	13.9	0.0	13.9	5.4	0.0	3.8	10.2	0.0	5.4
LnGrp LOS	B	A	A	B	A	B	A	A	A	B	A	A
Approach Vol, veh/h	144				248			775			758	
Approach Delay, s/veh	14.8				13.9			5.1			6.4	
Approach LOS	B				B			A			A	
Timer - Assigned Phs	2		4		6		8					
Phs Duration (G+Y+R _c), s	26.0		11.4		26.0		11.4					
Change Period (Y+R _c), s	4.5		4.5		4.5		4.5					
Max Green Setting (Gmax), s	33.0		18.0		33.0		18.0					
Max Q Clear Time (g_c+l1), s	9.7		6.5		16.9		4.7					
Green Ext Time (p_c), s	4.6		0.5		4.6		1.0					
Intersection Summary												
HCM 6th Ctrl Delay			7.5									
HCM 6th LOS			A									

Intersection

Intersection Delay, s/veh 72.3

Intersection LOS F

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	446	28	25	417	288	323
Future Vol, veh/h	446	28	25	417	288	323
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	1	1	2	2	2	2
Mvmt Flow	460	29	26	430	297	333
Number of Lanes	1	0	0	1	1	0
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		1		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	1		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	54.9		41.1		108.4	
HCM LOS	F		E		F	

Lane	NBLn1	EBLn1	SBLn1
Vol Left, %	6%	94%	0%
Vol Thru, %	94%	0%	47%
Vol Right, %	0%	6%	53%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	442	474	611
LT Vol	25	446	0
Through Vol	417	0	288
RT Vol	0	28	323
Lane Flow Rate	456	489	630
Geometry Grp	1	1	1
Degree of Util (X)	0.866	0.944	1.144
Departure Headway (Hd)	7.222	7.323	6.54
Convergence, Y/N	Yes	Yes	Yes
Cap	506	498	561
Service Time	5.222	5.323	4.54
HCM Lane V/C Ratio	0.901	0.982	1.123
HCM Control Delay	41.1	54.9	108.4
HCM Lane LOS	E	F	F
HCM 95th-tile Q	9.2	11.6	21.1

Intersection

Int Delay, s/veh 18.4

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	↗		↖
Traffic Vol, veh/h	37	389	302	203	475	324
Future Vol, veh/h	37	389	302	203	475	324
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	0	-	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	1	1	3	3
Mvmt Flow	39	409	318	214	500	341

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1659	318	0	0
Stage 1	318	-	-	-
Stage 2	1341	-	-	-
Critical Hdwy	6.42	6.22	-	4.13
Critical Hdwy Stg 1	5.42	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-
Follow-up Hdwy	2.2	2.2	-	2.227
Pot Cap-1 Maneuver	135	1039	-	1236
Stage 1	1116	-	-	-
Stage 2	318	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	68	1039	-	1236
Mov Cap-2 Maneuver	68	-	-	-
Stage 1	1116	-	-	-
Stage 2	159	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	63.8	0	5.9
HCM LOS	F		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	464	1236	-
HCM Lane V/C Ratio	-	-	0.966	0.405	-
HCM Control Delay (s)	-	-	63.8	9.9	0
HCM Lane LOS	-	-	F	A	A
HCM 95th %tile Q(veh)	-	-	12	2	-

Intersection

Int Delay, s/veh 1.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	N			
Traffic Vol, veh/h	7	31	411	11	53	263
Future Vol, veh/h	7	31	411	11	53	263
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	34	447	12	58	286

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	855	453	0	0	459
Stage 1	453	-	-	-	-
Stage 2	402	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	329	607	-	-	1102
Stage 1	640	-	-	-	-
Stage 2	676	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	308	607	-	-	1102
Mov Cap-2 Maneuver	308	-	-	-	-
Stage 1	640	-	-	-	-
Stage 2	633	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.6	0	1.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	515	1102	-
HCM Lane V/C Ratio	-	-	0.08	0.052	-
HCM Control Delay (s)	-	-	12.6	8.4	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.2	-

Intersection

Int Delay, s/veh 0.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	N			
Traffic Vol, veh/h	13	16	406	23	27	243
Future Vol, veh/h	13	16	406	23	27	243
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	14	17	441	25	29	264

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	776	454	0	0	466
Stage 1	454	-	-	-	-
Stage 2	322	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	366	606	-	-	1095
Stage 1	640	-	-	-	-
Stage 2	735	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	355	606	-	-	1095
Mov Cap-2 Maneuver	355	-	-	-	-
Stage 1	640	-	-	-	-
Stage 2	712	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	13.4	0	0.8
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	460	1095	-
HCM Lane V/C Ratio	-	-	0.069	0.027	-
HCM Control Delay (s)	-	-	13.4	8.4	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1	-

Intersection

Intersection Delay, s/veh 13.7

Intersection LOS B

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↓	↑	↑	↑	↑
Traffic Vol, veh/h	220	21	24	276	217	308
Future Vol, veh/h	220	21	24	276	217	308
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	5	5	4	4	3	3
Mvmt Flow	244	23	27	307	241	342
Number of Lanes	1	0	0	1	1	1
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		2		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	2		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	14.5		14.8		12.7	
HCM LOS	B		B		B	

Lane	NBLn1	EBLn1	SBLn1	SBLn2
Vol Left, %	8%	91%	0%	0%
Vol Thru, %	92%	0%	100%	0%
Vol Right, %	0%	9%	0%	100%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	300	241	217	308
LT Vol	24	220	0	0
Through Vol	276	0	217	0
RT Vol	0	21	0	308
Lane Flow Rate	333	268	241	342
Geometry Grp	5	2	7	7
Degree of Util (X)	0.525	0.461	0.394	0.492
Departure Headway (Hd)	5.672	6.197	5.885	5.175
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	637	581	610	695
Service Time	3.712	4.236	3.625	2.915
HCM Lane V/C Ratio	0.523	0.461	0.395	0.492
HCM Control Delay	14.8	14.5	12.4	12.9
HCM Lane LOS	B	B	B	B
HCM 95th-tile Q	3.1	2.4	1.9	2.7

Intersection

Int Delay, s/veh 6.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Vol, veh/h	57	420	450	72	215	231
Future Vol, veh/h	57	420	450	72	215	231
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	200	0	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	2	2	4	4
Mvmt Flow	60	442	474	76	226	243

Major/Minor	Minor1	Major1	Major2	
Conflicting Flow All	1169	474	0	0
Stage 1	474	-	-	-
Stage 2	695	-	-	-
Critical Hdwy	6.2	6.23	-	4.14
Critical Hdwy Stg 1	5.43	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-
Follow-up Hdwy	2.2	2.2	-	2.236
Pot Cap-1 Maneuver	306	830	-	1078
Stage 1	922	-	-	-
Stage 2	704	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	232	830	-	1078
Mov Cap-2 Maneuver	232	-	-	-
Stage 1	922	-	-	-
Stage 2	534	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	15.6	0	4.4
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	232	830	1078	-
HCM Lane V/C Ratio	-	-	0.259	0.533	0.21	-
HCM Control Delay (s)	-	-	25.8	14.2	9.2	0
HCM Lane LOS	-	-	D	B	A	A
HCM 95th %tile Q(veh)	-	-	1	3.2	0.8	-

Intersection

Intersection Delay, s/veh 36

Intersection LOS E

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↓	↑	↑	↑	↑
Traffic Vol, veh/h	446	28	25	417	288	323
Future Vol, veh/h	446	28	25	417	288	323
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles, %	1	1	2	2	2	2
Mvmt Flow	460	29	26	430	297	333
Number of Lanes	1	0	0	1	1	1
Approach	EB		NB		SB	
Opposing Approach			SB		NB	
Opposing Lanes	0		2		1	
Conflicting Approach Left	SB		EB			
Conflicting Lanes Left	2		1		0	
Conflicting Approach Right	NB			EB		
Conflicting Lanes Right	1		0		1	
HCM Control Delay	51.3		41.3		20.4	
HCM LOS	F		E		C	

Lane	NBLn1	EBLn1	SBLn1	SBLn2
Vol Left, %	6%	94%	0%	0%
Vol Thru, %	94%	0%	100%	0%
Vol Right, %	0%	6%	0%	100%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	442	474	288	323
LT Vol	25	446	0	0
Through Vol	417	0	288	0
RT Vol	0	28	0	323
Lane Flow Rate	456	489	297	333
Geometry Grp	5	2	7	7
Degree of Util (X)	0.875	0.935	0.61	0.618
Departure Headway (Hd)	6.913	6.891	7.398	6.678
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	524	528	487	539
Service Time	4.96	4.932	5.157	4.436
HCM Lane V/C Ratio	0.87	0.926	0.61	0.618
HCM Control Delay	41.3	51.3	21.1	19.7
HCM Lane LOS	E	F	C	C
HCM 95th-tile Q	9.6	11.5	4	4.2

Intersection

Int Delay, s/veh 7.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖ ↗ ↘ ↗ ↙ ↘					
Traffic Vol, veh/h	37	389	302	203	475	324
Future Vol, veh/h	37	389	302	203	475	324
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	Yield	-	None
Storage Length	200	0	-	50	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	1	1	3	3
Mvmt Flow	39	409	318	214	500	341

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	1659	318	0	0	318
Stage 1	318	-	-	-	-
Stage 2	1341	-	-	-	-
Critical Hdwy	5.7	6.22	-	-	4.13
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	2	3.318	-	-	2.227
Pot Cap-1 Maneuver	199	723	-	-	1236
Stage 1	1217	-	-	-	-
Stage 2	339	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	100	723	-	-	1236
Mov Cap-2 Maneuver	151	-	-	-	-
Stage 1	1217	-	-	-	-
Stage 2	170	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	18.1	0	5.9
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	151	723	1236	-
HCM Lane V/C Ratio	-	-	0.258	0.566	0.405	-
HCM Control Delay (s)	-	-	36.9	16.3	9.9	0
HCM Lane LOS	-	-	E	C	A	A
HCM 95th %tile Q(veh)	-	-	1	3.6	2	-

Queuing and Blocking Report
2024 Background plus Site Trips Mitigated - AM Peak

05/09/2021

Intersection: 1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (ft)	267	234	200	133	264	268	266	327	328	94	182	145
Average Queue (ft)	128	110	93	44	151	135	143	192	203	34	97	52
95th Queue (ft)	245	197	169	106	238	233	241	298	301	77	154	116
Link Distance (ft)		1207	1207		790	790		2255	2255		752	752
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	325			240			260			240		
Storage Blk Time (%)	1	0			1		2	1				0
Queuing Penalty (veh)	3	0			1		8	2				0

Intersection: 1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

Movement	SB
Directions Served	R
Maximum Queue (ft)	103
Average Queue (ft)	48
95th Queue (ft)	85
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	146
Storage Blk Time (%)	0
Queuing Penalty (veh)	0

Intersection: 2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

Movement	EB	WB	WB	NB	NB	SB	SB
Directions Served	LTR	LT	R	LT	R	L	TR
Maximum Queue (ft)	76	114	120	177	93	162	252
Average Queue (ft)	25	57	61	87	24	69	86
95th Queue (ft)	59	98	106	145	68	135	176
Link Distance (ft)	586	461		718			3808
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)		105		145	140		
Storage Blk Time (%)	0	0	0	0	1	1	
Queuing Penalty (veh)	1	1	0	0	4	2	

Queuing and Blocking Report
2024 Background plus Site Trips Mitigated - AM Peak

05/09/2021

Intersection: 3: SE 190th Avenue & SE Richey Road

Movement	EB	NB	SB	SB
Directions Served	LR	LT	T	R
Maximum Queue (ft)	100	104	139	114
Average Queue (ft)	46	60	53	59
95th Queue (ft)	80	95	96	90
Link Distance (ft)	1984	833	500	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)			200	
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 4: SE Foster Road & SE Richey Road

Movement	WB	WB	NB	NB	SB
Directions Served	L	R	T	R	LT
Maximum Queue (ft)	202	378	65	65	184
Average Queue (ft)	59	160	7	50	65
95th Queue (ft)	161	300	36	74	133
Link Distance (ft)		937	835		1150
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	200			50	
Storage Blk Time (%)	0	10	0	1	
Queuing Penalty (veh)	0	6	0	3	

Intersection: 5: SE 190th Avenue & North Site Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	64	43
Average Queue (ft)	28	3
95th Queue (ft)	53	19
Link Distance (ft)	603	833
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Queuing and Blocking Report
2024 Background plus Site Trips Mitigated - AM Peak

05/09/2021

Intersection: 6: SE 190th Avenue & South Site Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	54	36
Average Queue (ft)	26	2
95th Queue (ft)	48	15
Link Distance (ft)	598	394
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 29

Queuing and Blocking Report

2024 Background plus Site Trips Mitigated - PM Peak

05/09/2021

Intersection: 1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	TR	L	T	TR	L	T	TR	L	T	T
Maximum Queue (ft)	291	283	321	249	342	350	285	424	395	265	711	688
Average Queue (ft)	160	193	210	107	219	198	189	231	236	221	490	460
95th Queue (ft)	267	265	294	213	316	299	300	363	349	334	772	731
Link Distance (ft)	1207	1207			790	790		2255	2255		752	752
Upstream Blk Time (%)											4	2
Queuing Penalty (veh)											0	0
Storage Bay Dist (ft)	325			240			260			240		
Storage Blk Time (%)	0	0		0	5		5	4		5	42	52
Queuing Penalty (veh)	0	0		0	8		16	9		22	79	117

Intersection: 1: SE Highland Drive/SE 182nd Avenue & SE Powell Blvd

Movement	SB
Directions Served	R
Maximum Queue (ft)	171
Average Queue (ft)	133
95th Queue (ft)	225
Link Distance (ft)	
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	146
Storage Blk Time (%)	0
Queuing Penalty (veh)	2

Intersection: 2: SE 190th Avenue/SW Pleasant View Dr & SE Giese Road/SW Butler Road

Movement	EB	WB	WB	NB	NB	SB	SB	B15	B15
Directions Served	LTR	LT	R	LT	R	L	TR	T	
Maximum Queue (ft)	118	124	90	275	165	153	368	390	129
Average Queue (ft)	55	61	36	108	38	79	120	14	4
95th Queue (ft)	98	107	68	206	98	146	270	165	87
Link Distance (ft)	586	431		717			3808	2255	2255
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)			105		145	140			
Storage Blk Time (%)	1	0	2	0	3	2			
Queuing Penalty (veh)	1	0	3	0	15	3			

Queuing and Blocking Report
2024 Background plus Site Trips Mitigated - PM Peak

05/09/2021

Intersection: 3: SE 190th Avenue & SE Richey Road

Movement	EB	B16	NB	SB	SB
Directions Served	LR	T	LT	T	R
Maximum Queue (ft)	351	391	288	146	124
Average Queue (ft)	137	13	119	73	69
95th Queue (ft)	277	201	216	118	105
Link Distance (ft)	1984	937	863	510	
Upstream Blk Time (%)		0			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)			200		
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 4: SE Foster Road & SE Richey Road

Movement	WB	WB	NB	NB	SB
Directions Served	L	R	T	R	LT
Maximum Queue (ft)	124	206	66	65	303
Average Queue (ft)	38	96	20	56	107
95th Queue (ft)	88	168	61	62	224
Link Distance (ft)		937	835		1150
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	200			50	
Storage Blk Time (%)		0	0	2	
Queuing Penalty (veh)		0	1	6	

Intersection: 5: SE 190th Avenue & North Site Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	53	53
Average Queue (ft)	23	15
95th Queue (ft)	49	43
Link Distance (ft)	677	863
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Queuing and Blocking Report
2024 Background plus Site Trips Mitigated - PM Peak

05/09/2021

Intersection: 6: SE 190th Avenue & South Site Access

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	49	54
Average Queue (ft)	20	8
95th Queue (ft)	47	33
Link Distance (ft)	672	354
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 283

CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019

1 - 5 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S					
SER#	P	R	J	W	DATE	DIST	RNDBT	SURF	COLL	OWNER	FROM	G	E	LICNS	PED	ACT	EVENT	CAUSE		
INVEST	E	A	U	I	O	DAY	LEGS	TRAF-	SVRTY	# TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	
RD DFT	B	L	G	N	H	R	TIME	(#LANES)	CONT'L	DRWY	DRWY	STRTGT	UN - UN	29						
UNLOCK?	D	C	S	V	L	K	LAT	LRS	LOCTN	CROSS	N	0	0	0	0	0	0	0	0	
12145	N	N	N	11/03/2015	14	W POWELL BLVD	INTER	TRF SIGNAL	N	UNK	REAR	UNKN	UN - UN	0	0	0	0	0	0	0
NONE	TU	0		182ND AVE	UN		06	0	UN	UNK	INT	UNKNOWN	01	DRVR	NONE	0	M	UNK	026	
N	UNK			45 29 25 .22	-122 28														000	
N	6A			45 29 25 .22	-122 28														000	
				31.97															29	
02800	N	N	N	02/23/2016	14	W POWELL BLVD	INTER	CROSS	N	UNK	S-1STOP	01	NONE	9	STRTGT					000
NONE	TU	0		182ND AVE	UN		06	0	UN	UNK	REAR	N/A	UN - UN	0	0	0	0	0	0	0
N	4P			45 29 25 .22	-122 28														000	
N	4P			45 29 25 .22	-122 28														000	
				31.97															000	
03448	N	N	N	04/26/2018	14	W POWELL BLVD	INTER	CROSS	N	UNK	S-1STOP	01	NONE	9	STRTGT					000
NONE	TH	0		182ND AVE	UN		06	0	UN	UNK	RBAR	N/A	UN - UN	0	0	0	0	0	0	0
N	4P			45 29 25 .22	-122 28														000	
N	4P			45 29 25 .22	-122 28														000	
				31.97															000	
10781	N	N	N	11/21/2018	14	W POWELL BLVD	INTER	CROSS	N	UNK	S-1STOP	01	NONE	9	STRTGT					000
NONE	WE	0		182ND AVE	UN		06	0	UN	UNK	REAR	N/A	UN - UN	0	0	0	0	0	0	0
N	6P			45 29 25 .22	-122 28														000	
N	6P			45 29 25 .22	-122 28														000	
				31.97															000	
03048	Y	Y	N	N	03/22/2015	14	W POWELL BLVD	INTER	CROSS	N	CLR	0-1STOP	01	NONE	0	STRTGT				05,01
CITY	SU			0	182ND AVE		06	0	TRF SIGNAL	N	DRY	S - M	PRVTE	S	- N				000	
N	1A			45 29 25 .22	-122 28														05,01	
N	1A			45 29 25 .22	-122 28														05,01	
				31.97															000	

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
6 - 9 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	PRTC	INJ	A	S					
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RD/DBT	COLL	FROM	TO	G	E	LICNS	PED	ACT	EVENT	CAUSE	
INVEST	E	A	U	I	O	DAY	FROM	SECOND STREET	LEGS	SURF	P#	TYPE	SURVY	E	X	RES	LOC	ERROR		
RD DFT	B	L	G	N	H	R	TIME	LOCTN	(#LANES)	DRWY	STRTGHT	01	NONE	0					29	
UNLOC?	D	C	S	V	L	K	LAT	LRS	CONTL	DRWY	STRTGHT	01	NONE	0						
04239	N	N	N	04/22/2015	14	W	POWELL BLVD	INTER	CROSS	N	CLL	S-1STOP	01	NONE	0					00
NONE	WE	0		182ND AVE		N			TRP SIGNAL	N	DRY	REAR	PRVTE	N	-S					00
N	5P	45	29	25.22	-122.28	06	0			DAY	PDO		PSNGR CAR	01	DRV R	NONE	0	M	OR-Y OR<25	026
N				31.97								02	NONE	0						29
												PRVTE	PSNGR CAR	01	DRV R	NONE	4	F	OR-Y OR<25	000
												PSNGR CAR	01	DRV R	NONE	0				00
06946	N	N	N	06/30/2015	14	W	POWELL BLVD	INTER	CROSS	N	CLL	ANGL-STP	01	NONE	0					08
NO RPT	TU	0		182ND AVE		N			TRP SIGNAL	N	DRY	TURN	PRVTE	NW	-N					00
N	9A	45	29	25.22	-122.28	06	0			DAY	PDO		PSNGR CAR	01	DRV R	NONE	2	M	OR-Y OR<25	002
N				31.97								02	NONE	0						08
												PRVTE	PSNGR CAR	01	DRV R	NONE	5	F	OR-Y OR<25	000
												PSNGR CAR	01	DRV R	NONE	0				00
00051	N	N	N	01/03/2016	14	W	POWELL BLVD	INTER	CROSS	N	SNOW	S-1STOP	01	NONE	0					29
NONE	SU	0		182ND AVE		N			TRP SIGNAL	N	ICE	REAR	PRVTE	N	-S					00
N	2P	45	29	25.22	-122.28	06	0			DAY	INJ		PSNGR CAR	01	DRV R	NONE	4	M	OR-Y OR<25	026
N				31.97								02	NONE							29
												PRVTE	PSNGR CAR	01	DRV R	INJ/C	2	M	OR-Y OR<25	000
												PSNGR CAR	02	NONE	STOP	N	-S			00
												PRVTE	PSNGR CAR	02	PSNG	INJ/C	2	F		000
																			00	
15312	N	N	N	12/16/2016	14	W	POWELL BLVD	INTER	CROSS	N	CLD	S-1STOP	01	NONE	0					32,29
CITY	FR	0		182ND AVE		N			TRP SIGNAL	N	ICE	REAR	URKN	N	-S					00
N	12P	45	29	25.22	-122.28	06	0			DAY	INJ		PSNGR CAR	01	DRV R	NONE	1	M	None OR<25	052,026
N				31.97								02	NONE	0						00
												PRVTE	PSNGR CAR	01	DRV R	NONE	3	F	OR-Y OR<25	000
												PSNGR CAR	02	PSNG	INJ/C	2	F			00
																			00	
																			00	

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
10 - 13 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	PRTC	INJ	A	S		CAUSE		
SER#	P	R	J	W	DATE	DIST	RNDBT	SURF	COLL	OWNER	FROM	P#	TYPE	G	E	LICNS	PED	ACT	EVENT
INVEST	B	A	U	I	C	O	LEG'S	TRAF-	DRWY	VHTP	TO	SYRTY	E	X	RES	LOC	ERROR	013	01
RD DFT	B	L	G	N	H	R	LANES)	CONT'L											
UNLOCK?	D	C	S	V	L	K	LAT	LONG											
15860	Y	N	N		12/15/2016	14	W POWELL BLVD	INTER	CROSS	N									
NONE		TH	0		182ND AVE		N		TRF SIGNAL	N	ICE	REAR							
N		3P			45 29 25 .22	-122 28	31.97			06	0		DAY	INJ					
05416	N	N	N		05/10/2016	14	W POWELL BLVD	INTER	CROSS	N	CLR	O-1STOP	01 NONE	9	BACK				10
NONE		TU	0		182ND AVE		N		TRF SIGNAL	N	DRY	BACK							
N		6P			45 29 25 .22	-122 28	31.97			06	0		PDO						
12397	N	N	N		10/14/2016	14	W POWELL BLVD	INTER	CROSS	N	RAIN	S-1STOP	01 NONE	9	STRGHT				29
NONE		FR	0		182ND AVE		N		TRF SIGNAL	N	WET	REAR							
N		5P			45 29 25 .22	-122 28	31.97			05	0		DUSK	PDO					
00337	Y	N	N		01/11/2017	14	W POWELL BLVD	INTER	CROSS	N	FIX OBJ	01 NONE	0	TURN-R					
NO RPT		WE	0		182ND AVE		N		TRF SIGNAL	N	ICE	FIX							
N		10A			45 29 25 .22	-122 28	31.97			05	0		DAY	INJ					

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
14 - 16 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	PRTC	INJ	A	S		CAUSE		ACT	EVENT	013	01	
SER#	P	R	J	W	DATE	DIST	RNDBT	SURF	COLL	OWNER	FROM	P# TYPE	TYPE	G	E	LICNS	PED	ACT	EVENT	013	01		
INVEST	B	A	U	I	C	O	DAY	LEGS	DRWY	SVRTY	TO	P# TYPE	SVRTY	E	X	RES	LOC	ERROR		000	00		
RD DFT	B	L	G	N	H	R	TIME	(#LANES)	CONT'L											047,026	000	01	
UNLOC?	D	C	S	V	L	K	LAT	LRS	INTER	CROSS	N	CLR	S-1STOP	0	NON	0	STRGHT						
05228	Y	N	N	N	06/13/2019	14	W POWELL BLVD	N	TRP SIGNAL	DRY	REAR	PRVTE	N	-S							000	00	
CITY	TH	0			182ND AVE	06	0		DLIT	INJ	PSNGR CAR		01	DRVR	NONE	46	M	OR-Y OR<25			011,013	00	
N	10P				45 29 25 .22	-122 28	31.97					02	NONE	0	STOP	N	-S				000	00	
												PRVTE	PRVTE	INJ/C	57	M	OR-Y OR<25						
												PSNGR CAR		01	DRVR	INJ/C	36	M	OR-Y OR<25				
												PRVTE	PRVTE	INJ/C	27	F					022,013	00	
												PSNGR CAR		02	PSNG						000	00	
												PRVTE	PRVTE	INJ/C	27	F					022,013	00	
												PSNGR CAR		01	DRVR	NONE	42	M	OR-Y OR<25				
												PRVTE	PRVTE	INJ/C	27	F					000	00	
11764	N	N	N	N	11/21/2019	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-STRGHT	0	NON	9	STRGHT						13	
CITY	TH	0			182ND AVE	N			TRP SIGNAL	DRY	SS-O	N/A		N	-S							000	00
N	8P				45 29 25 .22	-122 28	31.97		DLIT	PDO	PSNGR CAR		01	DRVR	NONE	0	UNK	UNK			000	00	
												PSNGR CAR		02	NONE	9	STRGHT	N	-S				
												N/A		01	DRVR	NONE	0	UNK	UNK		000	00	
												PSNGR CAR		01	DRVR	NONE	0	UNK	UNK				
												PRVTE	PRVTE	INJ/C	27	F					000	00	
												PSNGR CAR		01	DRVR	NONE	42	M	OR-Y OR<25				
												PRVTE	PRVTE	INJ/C	27	F					000	00	
08718	N	N	N	N	08/13/2015	14	W POWELL BLVD	INTER	CROSS	N	CLR	PED	0	1	NON	0	TURN-R					02	
CITY	TH	0			182ND AVE	E	06	0	TRP SIGNAL	DRY	PED	PRVTE		E	-NW						016	00	
N	3P				45 29 25 .22	-122 28	31.97		DAY	INJ	PSNGR CAR		01	DRVR	NONE	48	F	NONE OR<25			029	000	
												PSNGR CAR		02	NONE	9	STRGHT	N	-S				
												PRVTE	PRVTE	INJ/C	27	F					000	00	
												PSNGR CAR		01	DRVR	NONE	0	UNK	UNK				
												PRVTE	PRVTE	INJ/C	27	F					000	00	
												PSNGR CAR		01	PED	INJ/B	26	F					
												PRVTE	PRVTE	INJ/C	27	F					035	00	
												PSNGR CAR		NW	SE								
08298	N	N	N	N	07/18/2016	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	0	1	NON	0	STRGHT					07,29	
CITY	MO				182ND AVE	E	06	0	TRP SIGNAL	DRY	REAR	PRVTE		E	-W						000	00	
N	2P				45 29 25 .22	-122 28	31.97		DAY	INJ	PSNGR CAR		01	DRVR	NONE	35	M	SUSP OR<25			043,026	000	

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
17 - 21 of 59 Crash records shown.

S	D	M	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTIC	INJ	A	S	CAUSE				
SER#	P	R J S W DATE	CLASS	FIRST STREET	TRAF- LEGS	RNDBT	SURF	COLL	OWNER	FROM	G	E	LICNS	PED	ACT EVENT				
INVEST	B	A U I C O	DAY	SECOND STREET	(#LANES)	DRWY	LIGHT	SURVY	V# TYP	TO	P# TYPE	SURVY	E	X	LOC	ERROR			
RD DFT	B	L G N H R	TIME	LONG	LOCTN	CONTL			02	NONE	STOP								
UNLOCK?	D	C S V L K	LAT	LRS					PRVTE	E -W	01 DRVR	INJ/C	39	F	OR-Y	000			
06151	N	N	N	07/01/2019	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	9	STRGHT	011	000		
CITY	MO	0		182ND AVE	E		TRF SIGNAL	N	DRY	REAR	N/A			E -W		000	00		
N	6P				06	0		N	DAY	PDO		PSNGR CAR			01 DRVR	000	000	00	
N	45 29 25 .22	-122 28									02	NONE	9	STOP				00	
											N/A	PSNGR CAR			E -W			00	
05461	N	N	N	05/28/2015	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT		000	00	
NO RPT	TH	0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR		PRVTE			SE -NW		000	00	
N	5P				06	0		N	DAY	INJ		PSNGR CAR			01 DRVR	026	000	29	
N	45 29 25 .22	-122 28									02	NONE	0	STOP				00	
											PUBLIC	OTH BUS			SE -NW			00	
10338	N	N	N	09/23/2015	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT		011	010	07.27
CITY	WE	0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR		PRVTE			SE -NW		000	00	
N	7A				06	0		N	DAY	INJ		PSNGR CAR			01 DRVR	026 , 043 , 016	038	07.27	
N	45 29 25 .22	-122 28									02	NONE	0	STOP				00	
											PRIVATE	MTRCYCLE			SE -NW			00	
08529	N	N	N	08/07/2017	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	9	STRGHT		011	010	29
NONE	MO	0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR	N/A			SE -NW		000	00		
N	12P				06	0		N	DAY	PDO		UNKNOWN			01 DRVR	000	000	00	
N	45 29 25 .22	-122 28									02	NONE	9	STOP				00	
											N/A	PSNGR CAR			SE -NW			00	
11342	N	N	N	10/15/2017	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	9	STRGHT		004	29	
NONE	SU	0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR	N/A			SE -NW		000	00		
N	10A				06	0		N	DAY	PDO		PSNGR CAR			01 DRVR	000	000	00	
N	45 29 25 .22	-122 28									02	NONE	0	STOP				00	
											N/A	PSNGR CAR			SE -NW			00	

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019

22 - 24 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S										
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RD CHAR	COLL	FROM	FRM	G	E	LICNS	PED									
INVEST	B	A	U	I	C	O	DAY	SECOND STREET	TRAF- LEGS	SURF	TO	P# TYPE	SVR TY	X	RES	LOC	ACT	EVENT	CAUSE						
RD DFT	B	L	G	N	H	R	TIME	LOCTN	DRWY	LIGHT	V# TYP E	02	NONE	9	STOP	SE-NW	011	013	0.0						
UNLOCK?	D	C	S	V	L	K	LAT	LRS	CONT'L		N/A	PSNGR CAR	01	DRV R	NONE	00	Unk UNK	000	000	0.0					
00479	N	N	N	N	01/19/2018	14	W POWELL BLVD	INTER	CROSS	N	RAIN	S-1STOP	01	NONE	0	STRGHT		29	29						
CITY		FR		0		182ND AVE	SE		TRF SIGNAL	N	WET	REAR	UNKN				SE-NW	000	000	0.0					
N		8P					06	0		N	DLIT	INJ	PSNGR CAR	01	DRV R	NONE	00	Unk UNK	026	000	29				
N		45	29	25	22	-122.28						0.2	NONE	0	STOP	SE-NW	011	000	0.0	0.0					
						31.97						PRVTE	PSNGR CAR	01	DRV R	INJ/C	47	M	OR-Y	000					
												0.2	NONE	0	STOP	SE-NW	02	PSNG	INJ/C	44	F				
												PRVTE	PSNGR CAR	02	PSNG	INJ/C	44	F	000	011	000				
																	000	000	0.0						
00519	N	N	N	N	01/20/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT		004	29						
NONE		SA		0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR	PRVTE				SE-NW	000	000	0.0					
N		12P					06	0		N	DAY	INJ	PSNGR CAR	01	DRV R	NONE	26	M	OR-Y	026	000	29			
N		45	29	25	22	-122.28						0.2	NONE	0	STOP	SE-NW	011	004	0.0	0.0					
						31.95						PRVTE	PSNGR CAR	01	DRV R	NONE	32	M	OR-Y	000	000				
												0.2	NONE	0	STOP	SE-NW	02	PSNG	INJ/C	25	F				
												PRVTE	PSNGR CAR	02	PSNG	INJ/C	25	F	000	011	004	0.0			
												0.2	NONE	0	STOP	SE-NW	011	004	0.0	0.0					
												PRVTE	PSNGR CAR	03	PSNG	NONE	01	M	000	011	004	0.0			
												0.2	NONE	0	STOP	SE-NW	04	FSNG	NONE	04	M	000	011	004	0.0
																	000	000	0.0						
03202	N	N	N	N	04/19/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT		013	07						
NO RPT		TH		0		182ND AVE	SE		TRF SIGNAL	N	DRY	REAR	PRVTE				SE-NW	000	000	0.0					
N		6A					06	0		N	DAY	INJ	PSNGR CAR	01	DRV R	NONE	62	M	OR-Y	026	000	0.7			
N		45	29	25	22	-122.28						0.2	NONE	0	STOP	SE-NW	011	013	0.0	0.0					
						31.97						PRVTE	PSNGR CAR	01	DRV R	INJ/C	43	M	OR-Y	000	000				

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
25 - 29 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S						
SER#	P	R	J	W	DATE	DIST	RD	CHAR	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED	ACT	EVENT	CAUSE	
03813	N	N	N	04/17/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	O-OTHER	01	NONE	9	TURN-L				0.8	0.8	
NONE		TU	0	182ND AVE	SE		TRF SIGNAL	N	DRY	TURN	N/A		N	-SE				0.0	0.0		
N		TA			05	0		DAY	PDO	PSNGR CAR		01	DRVR	NONE	00	Unk UNK		0.0	0.0		
N		45	29	25.22	-122	28						0.2	NONE	9	TURN-R				0.0	0.0	
				31.97								N/A	PSNGR CAR	01	DRVR	NONE	00	Unk UNK		0.0	
13036	N	N	N	07/21/2017	14	W POWELL BLVD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	TURN-R				0.2	0.2	
NONE		FR	0	182ND AVE	S		TRF SIGNAL	N	DRY	TURN	PRVTE		NW-S					0.0	0.0		
N		9P			05	0		DAY	DUSK	INJ	PSNGR CAR		01	DRVR	NONE	40	M	OR-Y	0.28		
N		45	29	25.22	-122	28						0.2	NONE	0	STRGHT				0.0	0.2	
				31.97								PRVTE	PSNGR CAR	01	DRVR	INJ/C	28	M	OR-Y	0.00	
06535	N	N	N	07/27/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT				0.29	0.29	
CITY		FR	0	182ND AVE	S		TRF SIGNAL	N	DRY	REAR	PRVTE		S-N					0.0	0.0		
N		8A			06	0		DAY	INJ	PSNGR CAR		01	DRVR	NONE	57	F	SUSP	043,026	0.29		
N		45	29	25.22	-122	28						0.2	NONE	0	STOP				0.0	0.0	
				31.97								PRVTE	PSNGR CAR	01	DRVR	INJ/C	40	F	OR-Y	0.00	
04295	N	N	N	05/16/2019	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-STRGHT	01	NONE	0	STRGHT				1.3	1.3	
COUNTY		TH	0	182ND AVE	S		UNKNOWN	N	DRY	SS-O	PRVTE		S-N					0.0	0.0		
N		7P			06	0		DAY	DUSK	INJ	PSNGR CAR		01	DRVR	NONE	00	M	UNK	0.45		
N		45	29	25.23	-122	28						0.2	NONE	0	STRGHT				0.0	0.0	
				32.01								PRVTE	PSNGR CAR	01	DRVR	INJ/C	36	M	OR-Y	0.00	
06878	N	N	N	06/14/2016	14	W POWELL BLVD	INTER	CROSS	N	CLD	PED	01	NONE	0	TURN-R				0.2	0.2	
CITY		TU	0	182ND AVE	W		TRF SIGNAL	N	DRY	PED	PRVTE		N-NW					0.0	0.0		
N		10A			05	0		DAY	INJ	PSNGR CAR		01	DRVR	NONE	16	M	N-VAL	0.29	0.0		
N		45	29	25.22	-122	28							PRVTE	PSNGR CAR	01	DRVR	INJ/C	36	M	OR-Y	0.00
				31.97													0.0	0.0			

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
34 - 37 of 59 Crash records shown.

S	D	M	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S	PED	ACT	EVENT	CAUSE
SER#	P	R J S W DATE	CLASS	FIRST STREET	RD DIRECT	RNDBT SURF	COLL	DRWY LIGHT	V# TYP	FROM	P# TYPE	SYRTY	E	X	RES	LOC	ERROR	
04353	N	N	04/14/2017	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-STRGHT	01	NONE	0	STRGHT				16,29
NONE		FR	0	182ND AVE	NW	TRF SIGNAL	N	DRY	REAR	PRVTE	NW-SE						000	00
N	N	6A	45 29 25 .22	-122 28	06	0	N	DAY	INJ	PSNGR CAR	01	DRVR	NONE	20	M	OR-Y OR<25	042	025
N	N	11A	45 29 25 .22	-122 28	31.97					02	NONE	0	STRGHT	NW-SE				16,29
05353	N	N	06/23/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT				29
NONE		SA	0	182ND AVE	NW	TRF SIGNAL	N	DRY	REAR	PRVTE	NW-SE						000	00
N	N	7P	45 29 25 .22	-122 28	06	0	N	DAY	INJ	PSNGR CAR	01	DRVR	NONE	32	M	OR-Y OR<25	026	000
N	N	11A	45 29 25 .22	-122 28	31.97					02	NONE	0	STOP	NW-SE				29
11335	N	N	N N 12/07/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT				013
CITY		FR	0	182ND AVE	NW	TRF SIGNAL	N	DRY	REAR	PRVTE	NW-SE						000	00
N	N	7P	45 29 25 .22	-122 28	06	0	N	DLTT	INJ	PSNGR CAR	01	DRVR	NONE	31	F	OTH-Y N-RES	026	000
N	N	11A	45 29 25 .22	-122 28	31.97					02	NONE	0	STOP	NW-SE				29
00604	N	N	N N 01/22/2019	14	W POWELL BLVD	INTER	CROSS	N	CLR	O-STRGHT	01	NONE	0	STRGHT				15
CITY		TU	0	182ND AVE	NW	TRF SIGNAL	N	DRY	HEAD	PRVTE	SE-NW						000	00
N	N	7A	45 29 25 .22	-122 28	05	0	N	DAY	INJ	PSNGR CAR	01	DRVR	NONE	44	M	OR-Y OR<25	039	000
N	N	11A	45 29 25 .22	-122 28	31.97					03	NONE	0	STOP	NW-SE				15

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
38 - 40 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S				
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RD CHAR	COLL	OWNER	FROM	G	E	LICNS	PED	ACT	EVENT	CAUSE
INVEST	E	A	U	I	O	DAY	FROM	SECOND STREET	RD CHAR	COLL	OWNER	FROM	P#	Type	SYRTY	E	X	RES	LOC
RD DFT	B	L	G	N	H	R TIME	TO	RD DFT	RD CHAR	COLL	OWNER	FROM	V#	TYP	SYRTY	E	X	RES	LOC
UNLOCK?	D	C	S	V	L	K LAT	LONG	LOCTN	RD CHAR	COLL	OWNER	FROM	02	NON E	0	STRAIGHT	NW-SE	000	000
LRS																			
11880	N	N	N	12/19/2019	14	W POWELL BLVD	INTER	CROSS	N	RAIN	S-1STOP	01	NON E	0	STRAIGHT	NW-SE	000	000	000
NO RPT	TH	0		182ND AVE	NW		06	0	WET	REAR	PRVTE	01	DRVR	NONE	19	F	OR-Y	026	000
N	9A	45	29	25.22	-122.28	31.97			DAY	INJ	PSNGR CAR	02	NON E	0	STOP	NW-SE	000	000	000
N	8P	45	29	25.22	-122.28	31.97					PSNGR CAR	02	NON E	0	STOP	NW-SE	000	000	000
											PSNGR CAR	03	NON E	0	STOP	NW-SE	000	000	000
											PSNGR CAR	01	DRVR	NONE	79	F	OR-Y	000	000
12232	N	N	N	12/29/2019	14	W POWELL BLVD	INTER	CROSS	N	CLR	S-1STOP	01	NON E	0	STRAIGHT	NW-SE	043	000	000
CITY	SU	0		182ND AVE	NW		06	0	DRY	REAR	PRVTE	01	DRVR	NONE	22	F	OR-Y	013	000
N	8P	45	29	25.22	-122.28	31.97			DLIT	INJ	PSNGR CAR	02	NON E	0	STOP	NW-SE	000	000	000
N	8P	45	29	25.22	-122.28	31.97					PSNGR CAR	02	NON E	0	STOP	NW-SE	000	000	000
											PSNGR CAR	03	NON E	0	STOP	NW-SE	000	000	000
											PSNGR CAR	01	DRVR	INJ	33	F	OR-Y	000	000
08016	N	N	N	07/27/2015	14	W POWELL BLVD	INTER	CROSS	N	CLR	O-1 L-TURN	01	NON E	0	STRAIGHT	NW-SE	000	000	000
NO RPT	MO	0		182ND AVE	CN		03	0	DRY	TURN	PRVTE	01	DRVR	NONE	42	F	OR-Y	000	000
N	11P	45	29	25.22	-122.28	31.97			DLIT	INJ	PSNGR CAR	02	NON E	0	STOP	NW-SE	000	000	000
N	11P	45	29	25.22	-122.28	31.97					PSNGR CAR	01	DRVR	INJ	17	F	OR-Y	000	000

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
45 - 48 of 59 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTC	INJ	A	S	CAUSE						
SER#			INVEST	E	A	U	I	O	DAY	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT SURF	COLL	OWNER	FROM	TO	P# TYPE	SYRTY	G	E	LICNS	PED	ACT	EVENT			
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOCNTN	(#LANES)	DRWY	LIGHT	SRTY	V# TYP	02 NONE	TO TURN-L	S -W	PRVTE	E	X	RES	LOC	ERROR	ACT	EVENT		
UNLOCK?	D	C	S	V	L	K	LAT	LONG	LRS	LOCTN	CONTLN	DRWY	LIGHT	SRTY	PRVTE	PSNGR CAR	02 FSNG	INJ/C	34 F	000	000	000	000	000	000	000		
15449	N	N	N	N	12/20/2016	14	W	POWELL BLVD	INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	0	STRGHT	N -S	PRVTE	INJ/C	18 M	OR-Y	OR<25	000	000	000	04		
CITY	TU	0			182ND AVE		CN		TRF SIGNAL	N	DRY	ANGL			PSNGR CAR		01 DRVR	INJ/C	18	M	OR-Y	OR<25	000	000	000			
N	11P	45	29	25	22	-122	28	31.97	03	0	DLTT	INJ			01 NONE	0	STRGHT	N -S	PRVTE	INJ/B	21	F		000	000	000		
N															PSNGR CAR		02 FSNG	INJ/B										
															01 NONE	0	STRGHT	N -S	PRVTE	INJ/B								
															PSNGR CAR		03 FSNG	INJ/B	17	F								
															02 NONE	0	STRGHT	NW -SE	PRVTE	INJ/C	33	M	OR-Y	OR<25	020	000	000	
															PSNGR CAR		01 DRVR	INJ/C										
00833	N	N	N	01/21/2016	14	W	POWELL BLVD	INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	9	TURN-R	N/A	S -SE									02		
NO RPT	TH	0		182ND AVE		CN		TRF SIGNAL	N	DRY	TURN	N/A														016	00	
N	1P	45	29	25	22	-122	28	31.97	04	0	DAY	PDO			PSNGR CAR		01 DRVR	NONE	00	Unk	UNK	UNK	000	000	000			
N															02 NONE	9	STRGHT	NW -SE	PRVTE	INJ/C								
															PSNGR CAR		01 DRVR	NONE	00	Unk	UNK	UNK	000	000	000			
09600	N	N	N	08/15/2016	14	W	POWELL BLVD	INTER	CROSS	N	CLR	S -OTHER	01 NONE	9	STRGHT	SE -NW									006	29		
NONE	MO	0		182ND AVE		CN		TRF SIGNAL	N	DRY	REAR	N/A				PSNGR CAR		01 DRVR	NONE	00	Unk	UNK	UNK	000	000	000		
N	12P	45	29	25	22	-122	28	31.97	02	0	DAY	PDO													00	00		
N															PSNGR CAR		02 NONE	9	STOP	SE -N								
															N/A		01 DRVR	NONE	00	Unk	UNK	UNK	000	000	000			

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
49 - 52 of 59 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	PRTC	INJ	A	S	CAUSE					
SER#	P	R	J	S	W	DATE	DI ST	RNDBT	SURF	OWNER	FROM	G	E	LICNS	PED						
INVEST	B	A	U	I	O	DAY	FIRST STREET	LEGS	COLL		TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT
RD DFT	B	L	G	N	H	R	FROM	(#LANES)	DRWY												
UNLOCK?	D	C	S	V	L	K	AT	CONTL	DRWY												
03511	N	N	N	N	04/06/2017	14	W POWELL BLVD	INTER	CROSS	N	RAIN	0-1 L-TURN	01	NONE	0	STRGHT				093	27,04
CITY	TH	0			182ND AVE		CN		TRF SIGNAL	N	WET	TURN				N -S				000	00
N	9P				45 29 25 .22	-122 28	01	0		N	DLIT	INJ				01 DRVR	NONE	3 6	F	016, 020	038 093
N					31.97											OR-Y	OR<25			27,04	
																				000	00
																				000	00
05289	N	N	N	N	05/19/2017	14	W POWELL BLVD	INTER	CROSS	N	CLR	O-1 L-TURN	01	NONE	0	TURN-L					04
CITY	FR	0			182ND AVE		CN		TRF SIGNAL	N	DRY	TURN				SE -S				000	00
N	9P				45 29 25 .22	-122 28	03	0		N	DLIT	INJ				01 DRVR	INJ	3 2	F	097, 004	000
N					31.97											OR-Y	OR<25			000	
																				000	00
																				000	00
12893	N	N	N	N	11/15/2017	14	W POWELL BLVD	INTER	CROSS	N	RAIN	ANGLE-OTH	01	NONE	9	TURN-R					04, 08
NONE	WE	0			182ND AVE		CN		TRF SIGNAL	N	WET	ANGL	N/A			N -NW				000	00
N	9A				45 29 25 .22	-122 28	01	0		N	DAY	PDO				01 DRVR	NONE	0 0	UNK UNK	000	000
N					31.97											UNK	UNK			000	
																				000	
																				000	
09606	N	N	N	N	10/19/2018	14	W POWELL BLVD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRGHT					02
NO RPT	FR	0			182ND AVE		CN		TRF SIGNAL	N	DRY	TURN				SE -NW				000	00
N	2P				45 29 25 .22	-122 28	01	0		N	DAY	INJ				01 DRVR	INJ	2 6	F	000	00
N					31.97										OR-Y	OR<25			016		
																			028	000	
																			02	00	

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
53 - 55 of 59 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FROM	PTRC	INJ	A	S	SPCL USE	TRLR QTY	OWNER	G	E	LICNS	PED	ACT	EVENT	CAUSE
N										FIRST STREET	DIRECT	TRAF-LEGS	RNDBT SURF	COLL																	
RD DFT	B	L	G	N	H	R	TIME			SECOND STREET	DIRECT	(#LANES)	CONTNL	DRWY	LIGHT	SURVY	# TYPE														
UNLOCK?	D	C	S	V	L	K	LAT			LRS	LOC TN																				
02632	N	N	N	03/15/2018	14	W	POWELL BLVD			INTER	CROSS	N	RAIN	O-1 L-TURN	01 NONE	9	STRGHT													02	
NONE				TH	0		182ND AVE			CN	TRF SIGNAL	N	WET	TURN	N/A		SE-NW												00		
N				8P				02	0		DARK	PDO					01 DRVR	NONE	00	Unk	UNK								00		
N				45	29	25	24	-122	28								02 NONE	9	TURN-L										00		
								31.98	31.97								N/A	PSNGR CAR		01 DRVR	NONE	00	Unk	UNK				00			
03445	N	N	N	N	04/23/2019	14	W	POWELL BLVD		INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	0	STRGHT												044.013		
CITY				TU	0		182ND AVE			CN	TRF SIGNAL	N	DRY	ANGL	PRVTE		NW-SE											04.27			
N				12A				03	0		DLTT	INJ					02 NONE	0	STRGHT	N-S		01 DRVR	INJ/C	26	F	OR-Y		020.016			
N				45	29	25	22	-122	28								03 NONE	0	TURN-L	N-SE		01 DRVR	INJ/C	24	F	OR-Y		000			
									31.97								03 NONE	0	TURN-L	N-SE		01 DRVR	INJ/C	24	F	OR-Y		000			
																	03 NONE	0	TURN-L	N-SE		01 DRVR	INJ/C	30	M			038			
00757	N	N	N	01/28/2019	14	W	POWELL BLVD			INTER	CROSS	N	CLR	O-1 L-TURN	01 NONE	0	STRGHT												04		
NONE				MO	0		182ND AVE			CN	TRF SIGNAL	N	DRY	TURN	PRVTE		S-N											00			
N				6P				04	0		DLTT	INJ					PSNGR CAR				01 DRVR	NONE	68	F	OR-Y		020				
N				45	29	25	22	-122	28								02 NONE	0	TURN-L	N-SE		01 DRVR	INJ/C	53	M	OR-Y		000			
									31.97								PSNGR CAR										00				

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CITY OF GRESHAM, MULTNOMAH COUNTY

POWELL BLVD at 182ND AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019

56 - 59 of 59 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FRTIC	INJ	A	S	ACT	EVENT	CAUSE			
SER#									DIST	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	OWNER	FROM	G	E	LICNS	PED	LOC	ERROR				
INVEST	E	A	U	I	C	O	DAY		FROM	SECOND STREET	INDCTN	(#LANES)	CONTL	DRWY	LIGHT	SVRTY	V# TYPE	P# TYPE	SVRTY	E	X	RES					
RD DFT	B	L	G	N	H	R	TIME		LRS	LONG	LOCTN																
UNLOCK?	D	C	S	V	L	K	LAT																				
10620	N	N	N	11/14/2019	14	W	POWELL BLVD		INTER	CROSS	N		CLR	ANGL-OTH	01 NONE	0	STRGHT								04		
NO RPT									CN	TRF SIGNAL	N		DRY	ANGL			PRVTE										
	TH	0		182ND AVE					03	0			DAWN	INJ			PSNGR CAR			01 DRVR	NONE	65	F	OR-Y OR<25	020	000	04
N				6A																							
				45 29 25.22	-122	28																					
							31.97																				
11853	N	N	N	12/18/2019	14	W	POWELL BLVD		INTER	CROSS	N		RAIN	O-OTHER	01 NONE	0	TURN-L									02,08,04	
CITY				WE	0				CN	TRF SIGNAL	N		WET	TURN			UNKN			NW-N						00	
N				11P					02	0			DLIT	INJ			PSNGR CAR			01 DRVR	NONE	00	Unk	UNK	028,001,020	000	02,08,04
				45 29 25.22	-122	28																					
							31.97																				
11765	N	N	N	11/21/2019	14	W	POWELL BLVD		INTER	CROSS	N		CLR	O-1 L-TURN	01 NONE	9	TURN-L									02,27	
CITY				TH	0				CN	TRF SIGNAL	N		DRY	TURN			N/A			W-N						00	
N				6P					02	0			DLIT	PDO			PSNGR CAR			01 DRVR	NONE	00	Unk	UNK	000	000	00
				45 29 25.25	-122	28																					
							31.96																				
84391	N	N	N	12/06/2019	14	W	POWELL BLVD		INTER	CROSS	N		RAIN	ANGLE-OTH	01 NONE	9	TURN-R									08,02	
NONE				FR	0				CN	TRF SIGNAL	N		WET	TURN			N/A			E-N						016	
N				8P					02	0			DLIT	PDO			PSNGR CAR			01 DRVR	NONE	00	Unk	UNK	000	000	00
				45 29 25.22	-122	28																					
							31.97																				

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97215 to 12/31/2019
1 - 5 of 23 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE			
SER#			INVEST	E	A	U	I	O	DAY	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	FROM	G	LICNS	PED	ACT	EVENT			
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOCNTN	(#LANES)	CONTNL	DRWY	LIGHT	SURVY	TO	P# TYPE	SVRTY	E	X	RES	LOC	ERROR	
UNLOC?	D	C	S	V	L	K	LAT	LONG	LRS	INTER	CROSS	N	CLR	S-STRGHT	01 NONE	9	STRGHT						01,27,29	
11697	Y	N	N	10/03/2016	16	SE BUTLER RD	N	SE 190TH DR	N	INTER	CROSS	N	RAIN	S-1STOP	01 NONE	9	STRGHT						124,129	29
NONE	MO	0	SE 190TH DR	06	0	STOP SIGN	N	DRY	REAR	N/A	N/A	N	-S									000	00	
N	5P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	INTER	CROSS	N	ICE	REAR	N/A	N/A	01 DRV	NONE	00	Unk	Unk	000	00	
N	5P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	TRF SIGNAL	N	DLIT	PDO	PSNGR CAR			01 DRV	NONE	00	Unk	Unk	000	00	
14479	N	N	N	12/25/2017	16	SE BUTLER RD	N	SE 190TH DR	N	INTER	CROSS	N	RAIN	S-1STOP	01 NONE	9	STRGHT						124,129	29
NONE	MO	0	SE 190TH DR	06	0	STOP SIGN	N	DRY	REAR	N/A	N/A	N	-S									000	00	
N	9P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	INTER	CROSS	N	ICE	REAR	N/A	N/A	01 DRV	NONE	00	Unk	Unk	000	00	
N	9P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	TRF SIGNAL	N	DLIT	PDO	PSNGR CAR			01 DRV	NONE	00	Unk	Unk	000	00	
10273	Y	N	N	08/30/2016	16	SE BUTLER RD	E	SE 190TH DR	E	INTER	CROSS	N	UNK	FIX OBJ	01 NONE	9	TURN-L						040,058	32,01,08
CITY	TU	0	SE 190TH DR	05	0	STOP SIGN	N	DRY	FIX	N/A	N/A	N	-E									000	00	
N	2A	45	28	20.93	-122	28	1.3	SE 190TH DR	05	INTER	CROSS	N	DRY	REAR	N/A	N/A	01 DRV	NONE	00	Unk	Unk	000	00	
N	2A	45	28	20.93	-122	28	1.3	SE 190TH DR	05	TRF SIGNAL	N	DLIT	PDO	PSNGR CAR			01 DRV	NONE	00	Unk	Unk	000	00	
10909	N	N	N	08/06/2015	16	SE BUTLER RD	S	SE 190TH DR	S	INTER	CROSS	N	CLR	S-STRGHT	01 NONE	0	STRGHT						07	
NONE	TH	0	SE 190TH DR	06	0	STOP SIGN	N	DRY	REAR	PRVTE	PRVTE	S	-N									000	00	
N	4P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	INTER	CROSS	N	DAY	PDO	PSNGR CAR			01 DRV	NONE	24	F	OR-Y OR<25	042	000
N	4P	45	28	20.93	-122	28	1.3	SE 190TH DR	06	TRF SIGNAL	N	DLIT	PDO	PSNGR CAR			02 NONE	0	STRGHT	S	-N	000	00	
11098	Y	N	N	10/10/2015	17	SE BUTLER RD	CN	SE 190TH DR	04	INTER	CROSS	N	RAIN	ANGL-OTH	01 NONE	0	STRGHT						124	01,02
CITY	SA	0	SE 190TH DR	04	0	STOP SIGN	N	WET	ANGL	PRVTE	PRVTE	S	-N									000	00	
N	3P	45	28	20.93	-122	28	1.3	SE 190TH DR	04	INTER	CROSS	N	DAY	INJ	PSNGR CAR			01 DRV	INJ	45	F	OR-Y OR<25	047,028	000
N	3P	45	28	20.93	-122	28	1.3	SE 190TH DR	04	TRF SIGNAL	N	DLIT	PDO	PSNGR CAR			02 NONE	0	STRGHT	W	-E	000	00	

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97211-7200 to 12/31/2019
6 - 10 of 23 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S						
SER#	P	R	J	W	DATE	DI ST	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED	ACT	EVENT	CAUSE			
INVEST	E	A	U	I	O	FIRST STREET	LEGS	DRWY	LIGHT	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR		
RD DFT	B	L	G	N	H	FROM	CTRL	DRWY	ANGLE	STRGHT										
UNLOC?	D	C	S	V	L	K	LANES)	CTRL	ANGLE-OTH	01	NONE	0								
09129	N	N	N	08/24/2015	16	SE BUTLER RD	INTER	CROSS	N	CLR	PRVTE	S	-N					02		
	MO	0		SE 190TH DR	CN			STOP SIGN	N	DRY	PSNGR CAR	01	DRVR	62	F	OR-Y	OR<25	0.2		
N		1P		45 28 20 .93	-122 28	1.3	04	0		DAY	PDO			00	000	000	000	0.0		
												02	NONE	0	STRGHT	W -E	015	0.0		
											PSNGR CAR	PRVTE	01	DRVR	18	F	OR-Y	OR<25	0.2	
09208	N	N	N	08/26/2015	16	SE BUTLER RD	INTER	CROSS	N	CLR	PRVTE	N	-S					0.2		
	WE	0		SE 190TH DR	CN			STOP SIGN	N	DRY	PSNGR CAR	01	DRVR	68	F	OR-Y	OR<25	0.0		
N		8A		45 28 20 .93	-122 28	1.3	01	0		DAY	PDO			00	000	000	000	0.0		
											PSNGR CAR	PRVTE	02	NONE	0	STRGHT	E -W	015	0.0	
											PSNGR CAR	PRVTE	01	DRVR	16	M	OR-Y	OR<25	0.2	
11222	N	N	N	09/21/2016	16	SE BUTLER RD	INTER	CROSS	N	CLR	PRVTE	S	-N					0.2		
CITY		WE	0	SE 190TH DR	CN			STOP SIGN	N	DRY	PSNGR CAR	01	DRVR	41	M	OTH-Y	N/RES	0.0		
N		3P		45 28 20 .93	-122 28	1.3	04	0		DAY	INJ			02	NONE	0	STRGHT	W -E	015	0.0
											PSNGR CAR	PRVTE	02	NONE	0	STRGHT	E -W	000	0.2	
											PSNGR CAR	PRVTE	01	DRVR	24	M	OR-Y	OR<25	0.0	
15838	N	N	N	12/31/2016	16	SE BUTLER RD	INTER	CROSS	N	CLR	PRVTE	N	-S					0.2		
CITY		SA	0	SE 190TH DR	CN			STOP SIGN	N	DRY	PSNGR CAR	01	DRVR	43	F	OR-Y	OR<25	0.0		
N		5P		45 28 20 .93	-122 28	1.3	01	0		DLIT	INJ			02	NONE	0	STRGHT	E -W	015	0.0
											PSNGR CAR	PRVTE	02	NONE	0	STRGHT	E -W	000	0.2	
											PSNGR CAR	PRVTE	01	DRVR	32	M	SUSP	OR<25	0.0	
09949	N	N	N	08/22/2016	16	SE BUTLER RD	INTER	CROSS	N	CLR	PRVTE	N	-S					0.2		
	MO	0		SE 190TH DR	CN			STOP SIGN	N	DRY	PSNGR CAR	01	DRVR	0	UNK	UNK	UNK	0.0		
N		10A		45 28 20 .93	-122 28	1.3	01	0		DAY	PDO			02	NONE	9	STRGHT	N -S	000	0.0
											PSNGR CAR	PRVTE	01	DRVR	0	UNK	UNK	UNK	0.0	

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97210 to 12/31/2019
11 - 14 of 23 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	PRTC	INJ	A	S		CAUSE						
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RNDBT SURF	COLL	FROM	G	E	LICNS	PED							
INVEST	B	A	U	I	C	O	DAY	SECOND STREET	LEGS	DRWY	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	
RD DFT	B	L	G	N	H	R	TIME	LRS	(#LANES)	CONT'L												
UNLOCK?	D	C	S	V	L	K	LAT	LONG														
06248	N	N	N	06/13/2017	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	TURN-R					03		
NONE	TU	0	SE 190TH DR	CN	04	0	STOP SIGN	N	DRY	TURN	PRVTE	S	-E						000	00		
N	4P	45	28	20	.93	-122	28	1.3											000	00	03	
13404	N	N	N	12/02/2017	16	SE BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT					02		
CITY	SA	0	SE 190TH DR	CN	02	0	STOP SIGN	N	WET	ANGL	PRVTE	E	-W						000	00		
N	7A	45	28	20	.93	-122	28	1.3											028	000	02	
05573	N	N	N	05/26/2017	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT					02		
NO RPT	FR	0	SE 190TH DR	CN	02	0	STOP SIGN	N	DRY	ANGL	N/A	E	-W						015	00		
N	2P	45	28	20	.93	-122	28	1.3											000	00		
08027	N	N	N	07/26/2017	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT					03		
NONE	WE	0	SE 190TH DR	CN	04	0	STOP SIGN	N	DRY	ANGL	N/A	S	-N						000	00		
N	1P	45	28	20	.93	-122	28	1.3											000	00		

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97210 from 01/01/2015 to 12/31/2019
15 - 17 of 23 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FROM	PRTC	INJ	A	S		CAUSE				
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RD/DBT	SURF	OWNER	P#	TYP E	G	E	LICNS	PED	ACT	EVENT		
INVEST	B	A	U	I	C	O	DAY	SECOND STREET	LEGS	COLL		TYPE	SYRVTY	E	X	RES	LOC	ERROR			
RD DFT	B	L	G	N	H	R	TIME	LRS	(#LANES)	DRWY											
UNLOCK?	D	C	S	V	L	K	LAT	LONG	CONT'L												
14121	N	N	N	12/18/2017	16	SE BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	9	STRGHT				03		
NONE	MO	0	SE 190TH DR	CN	03	0	STOP SIGN	N	WET	ANGL	N/A			N -S				000	00		
N	N	4P	45 28 20 .93	-122 28	1.3				DUSK	PDO	PSNGR CAR			01	DRV R	NONE	00	Unk UNK	000		
												02	NONE	9	STRGHT						
											N/A	PSNGR CAR		W -E	01	DRV R	NONE	00	Unk UNK	000	
																		000	00		
00523	N	N	N	01/20/2018	16	SE BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT				02,03		
NO RPT	SA	0	SE 190TH DR	CN	03	0	STOP SIGN	N	WET	ANGL	PRVTE			N -S				000	00		
N	N	4P	45 28 20 .93	-122 28	1.3				DAY	INJ	PSNGR CAR			01	DRV R	NONE	72	M OR-Y	028,021		
											02	NONE	0	STRGHT							
											PRVTE	PSNGR CAR		W -E	01	DRV R	INJ/C	25	F OR-Y	000	
											02	NONE	0	STRGHT					000	00	
											PRVTE	PSNGR CAR		W -E	02	PSNG	INJ/C	28	F	000	00
											02	NONE	0	STRGHT							
											PRVTE	PSNGR CAR		W -E	03	PSNG	INJ/B	13	F	000	00
																		000	00		
05939	N	N	N	07/11/2018	17	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRGHT				02,03		
NONE	WE	0	SE 190TH DR	CN	04	0	STOP SIGN	N	DRY	ANGL	PRVTE			S -N				000	00		
N	N	3P	45 28 20 .93	-122 28	1.3				DAY	INJ	PSNGR CAR			01	DRV R	NONE	31	F OR-Y	028,021		
											01	NONE	0	STRGHT							
											PRVTE	PSNGR CAR		S -N	02	PSNG	NONE	04	F	000	00
											01	NONE	0	STRGHT							
											PRVTE	PSNGR CAR		S -N	03	PSNG	NONE	08	M	000	00
											02	NONE	0	STRGHT							
											PRVTE	PSNGR CAR		W -E	01	DRV R	INJ/C	41	M OR-Y	000	00
																		000	00		

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97210 to 12/31/2019
18 - 22 of 23 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	PRTC	INJ	A	S			CAUSE		
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RNDBT SURF	COLL	FROM	G	E	LICNS	PED	LOC	ERROR	ACT	EVENT
INVEST	E	B	A	U	I	O	DAY	SECOND STREET	LEGS	DRWY	TO	P#	TYPE	SVRTY	E	X	RES		
RD DFT	B	L	G	N	H	R	TIME	LOCTN	(#LANES)	CONT'L									
UNLOCK?	D	C	S	V	L	K	LAT	LRS											
04941	N	N	N	06/11/2018	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT			02,03	
NONE	MO	0		SE 190TH DR	CN			STOP SIGN	N	DRY	ANGL	N/A		S -N				000	00
N	11A	45	28	20.93	-122	28	04	0		DAY	PDO	PSNGR CAR		01	DRV R	NONE	00	Unk UNK	000
N	1.31	1.3																000	00
05220	N	N	N	06/19/2018	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	TURN - L			02,03	
NONE	TU	0		SE 190TH DR	CN			STOP SIGN	N	DRY	TURN	N/A		W -N				000	00
N	12P	45	28	20.92	-122	28	04	0		DAY	PDO	PSNGR CAR		01	DRV R	NONE	00	Unk UNK	000
N	1.31																	000	00
00359	N	N	N	01/10/2019	16	SE BUTLER RD	INTER	CROSS	N	CLD	ANGL-OTH	01	NONE	0	TURN - L			02,04	
NONE	TH	0		SE 190TH DR	CN			STOP SIGN	N	WET	TURN	PRVTE		W -N				000	00
N	7A	45	28	20.93	-122	28	04	0		DAWN	INJ	PSNGR CAR		01	DRV R	NONE	51	F OR-Y OR<25	028,004,021
N	1.3	1.3																000	00
04056	N	N	N	05/10/2019	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRGHT			03	
CITY	FR	0		SE 190TH DR	CN			STOP SIGN	N	DRY	ANGL	PRVTE		S -N				000	00
N	11A	45	28	20.93	-122	28	04	0		DAY	INJ	PSNGR CAR		01	DRV R	NONE	64	F OR-Y OR<25	021
N	1.3	1.3																000	03
11848	N	N	N	12/18/2019	17	SE BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT			27,02,03	
CITY	WE	0		SE 190TH DR	CN			STOP SIGN	N	WET	ANGL	PRVTE		N -S				000	00
N	5P	45	28	20.93	-122	28	01	0		DLIT	INJ	PSNGR CAR		01	DRV R	INJ B	32	F OR-Y OR<25	016,028,021
N	1.3	1.3																038	27,02,03

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CITY OF GRESHAM, MULTNOMAH COUNTY

190TH DR at BUTLER RD, City of Gresham, Multnomah County, OR 97215 to 12/31/2019

23 - 23 of 23 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A	S	
INVEST	E	A	U	I	C	O	DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF- (#LANES)	RNDBT	SURF	TRLR QTY	FROM	PRTC	INJ	G	
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOCTN	CONTL	DRWY	LIGHT	SVRTY	V# TYP	TO	P# TYPE	SVRTY	E	
04075	N	N	N	N	N	N	N	05/11/2019	16	SE BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT	02,03
CITY	SA	0		SE 190TH DR	CN				STOP SIGN	N	DRY	ANGL	N/A		S -N					00
N	8A	45	28	20	.93	-1.22	.28		04	0			N	DAY	PDO		01	DRVR	NONE	0.0
N		1.3																Unk	Unk	0.0
																				0.0
																				0.0
																				0.0

CITY OF GRESHAM, MULTNOMAH COUNTY

GISE RD at 190TH AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
1 - 4 of 7 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	PRTC	INJ	A	S		CAUSE			
SER#	P	R	J	S	W	DATE	DI ST	FIRST STREET	RNDBT SURF	COLL	FROM	G	E	LICNS	PED				
INVEST	B	A	U	I	C	O	DAY	SECOND STREET	LEGS	DRWY	TO	P#	TYPE	SVRTY	E	RES	LOC	ERROR	
RD DFT	B	L	G	N	H	R	TIME	LRS	(#LANES)	CONT'L									
UNLOCK?	D	C	S	V	L	K	LAT	LONG											
12094	N	N	N	N	11/02/2015	19	SE	GISE RD	INTER	CROSS	N	CLD	S-1STOP	01	NONE	0	STRGHT	07	
CITY	MO	0	SE	190TH AVE	W	06	0	STOP SIGN	N	WET	REAR	PRVTE		W	-E		000	00	
N	N	7A	45	28	20.93	-122	28	1.3	04	0	DAY	INJ	PSNGR CAR		01	DRVR	NONE	00	026, 043
												02	NONE	0	STOP	W	-E	011	
												PRVTE	PSNGR CAR		01	DRVR	INJ/C	4.6	M
												02	NONE	0	STOP	W	-E	000	
												PRVTE	PSNGR CAR		02	PSNG	INJ/C	27	M
06259	N	N	N	06/15/2015	16	SE	GISE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRGHT	02		
NONE	MO	0	SE	190TH AVE	CN	04	0	STOP SIGN	N	DRY	ANGL	PRVTE		S	-N		015	00	
N	N	8P	45	28	20.93	-122	28	1.3	04	0	DAY	PDO	PSNGR CAR		01	DRVR	NONE	00	000
												02	NONE	0	STRGHT	W	-E	000	
												PRVTE	PSNGR CAR		01	DRVR	NONE	6.1	F
												02	PSNG			0.28		0.28	
08897	N	N	N	08/17/2015	17	SE	GISE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRGHT	02		
NONE	MO	0	SE	190TH AVE	CN	04	0	STOP SIGN	N	DRY	ANGL	PRVTE		W	-E		015	00	
N	N	6P	45	28	20.93	-122	28	1.3	04	0	DAY	PDO	PSNGR CAR		01	DRVR	NONE	2.6	M
												02	NONE	0	STRGHT	S	-N	028	
												PRVTE	PSNGR CAR		01	DRVR	NONE	6.5	M
												02	PSNG			0.00		0.00	
11511	N	N	N	10/19/2017	16	SE	GISE RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT	124		
NONE	TH	0	SE	190TH AVE	CN	02	0	STOP SIGN	N	WET	ANGL	PRVTE		S	-N		000	124	
N	N	1P	45	28	20.93	-122	28	1.3	02	0	DAY	INJ	PSNGR CAR		01	DRVR	NONE	3.4	F
												02	NONE	0	STRGHT	E	-W	021	
												PRVTE	PSNGR CAR		01	DRVR	INJ/C	27	M
												02	NONE	0	STRGHT	E	-W	000	
												PRVTE	PSNGR CAR		02	PSNG	INJ/C	0.0	M

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CITY OF GRESHAM, MULTNOMAH COUNTY

GIESE RD at 190TH AVE, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
5 - 7 of 7 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FROM	FRTC	INJ	A	S	ACT	EVENT	CAUSE				
SER#									DIST	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	OWNER	P# TYPE	TYPE	TO	P# TYPE	SVRTY	G	E	LICNS	PED	LOC	ERROR			
UNLOC?	D	C	S	V	L	K	LAT	LONG	LRS	SECOND STREET	LOCTN	(#LANES)	CONTL	DRWY	LIGHT														
01677	N	N	N	02/15/2017	16	SE	GIESE RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	9	STRGHT											02	0.2	
NO RPT	WE	0	SE 190TH AVE	CN	STOP	SIGN	N	WET	ANGL	N/A	N/A	S -N															0.0		
N	7P	45 28 20 .93	-122 28	04	0	DLIT	PDO	PSNGR CAR		01	DRVR	NONE	0.0	Unk	Unk	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
N	1.3	1.3																											
03152	N	N	N	04/17/2018	16	SE	GIESE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT												0.2	0.03
NONE	TU	0	SE 190TH AVE	CN	STOP	SIGN	N	DRY	ANGL	N/A	N/A	N -S															0.0		
N	3P	45 28 20 .93	-122 28	03	0	DAY	PDO	PSNGR CAR		01	DRVR	NONE	0.0	Unk	Unk	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
N	1.3	1.3																											
06994	N	N	N	08/08/2018	16	SE	GIESE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT												0.2	
NONE	WE	0	SE 190TH AVE	CN	STOP	SIGN	N	DRY	ANGL	N/A	N/A	N -S															0.0		
N	5P	45 28 20 .93	-122 28	03	0	DAY	PDO	PSNGR CAR		01	DRVR	NONE	0.0	Unk	Unk	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
N	1.3	1.3																											

CITY OF GRESHAM, MULTNOMAH COUNTY

BUTLER RD at PLEASANT VIEW DR, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
1 - 5 of 5 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	PRTC	INJ	A	S	CAUSE					
SER#	P	R	J	W	DATE	DIST	RNDBT	SURF	COLL	OWNER	FROM	P#	TYPE	G	E	LICNS	PED	ACT	EVENT		
INVEST	B	A	U	I	O	DAY	LEGS	DRWY	LIGHT	SVRTY	TO	SVRTY	E	X	RES	LOC	ERROR				
RD DFT	B	L	G	N	H	R	TRAF- (#LANES)	CONTL											07/01		
UNLOC?	D	C	S	V	L	K	LAT	LRS													
07636	Y	N	N	N	07/01/2016	16	SW BUTLER RD	INTER	CROSS	N	CLR	S-STOP	01	NONE	9	STRGHT				00	
CITY	FR	0			SW PLEASANT VIEW DR	E			STOP SIGN	N	DRY	REAR	N/A			W -E				00	
N	2P				45 28 20 .93	-122 28				N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	00	00	
N					1.3												Unk UNK	00		00	
01700	Y	N	N	N	02/08/2017	16	SW BUTLER RD	INTER	CROSS	N	RAIN	FIX OBJ	01	NONE	9	TURN - L				040	32,01,08
NO RPT	WE	0			SW PLEASANT VIEW DR	E			STOP SIGN	N	WET	FIX	N/A			N -E				000	00
N	1A				45 28 20 .93	-122 28				N	DLIT	PDO	PSNGR CAR			01	DRVR	NONE	00	000	00
04744	N	N	N	N	04/24/2016	17	SW BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT					02
CITY	SU				SW PLEASANT VIEW DR	CN			STOP SIGN	N	WET	ANGL	PRVTE			W -E				015	00
N	3P				45 28 20 .93	-122 28				N	DAY	INJ	PSNGR CAR			01	DRVR	INJ	18	028	02
N					1.3											PRVTE	PSNGR CAR			00	
80968	N	N	N	N	02/29/2016	16	SW BUTLER RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	9	STRGHT					02
NONE	MO	0			SW PLEASANT VIEW DR	CN			STOP SIGN	N	WET	ANGL	N/A			S -N				015	00
N	8A				45 28 20 .93	-122 28				N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	00	000	00
N					1.3											PRVTE	PSNGR CAR			015	00
06156	N	N	N	N	05/27/2016	16	SW BUTLER RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT					02
NONE	FR	0			SW PLEASANT VIEW DR	CN			STOP SIGN	N	UNK	ANGL	N/A			S -N				000	00
N	5P				45 28 20 .93	-122 28				N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	00	000	00
N					1.3											PRVTE	PSNGR CAR			015	00

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CITY OF GRESHAM, MULTNOMAH COUNTY

GIESE RD at PLEASANT VIEW DR, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019
1 - 4 of 4 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FROM	FRTC	INJ	A	S					
SER#	P	R	J	W	DATE	DIST	RNDBT	SURF	COLL	OWNER	TO	P# TYPE	SVRTY	G	E	LICNS	PED	ACT	EVENT	CAUSE
INVEST	B	A	U	I	O	DAY	LEGS	DRWY	LIGHT	V#	P#	TYPE	SVRTY	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
RD DFT	B	L	G	N	H	R	TIME	(#LANES)	CONTL											02
UNLOCK?	D	C	S	V	L	K	LAT	LRS	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT			
09166	N	N	N	N	08/06/2016	16	SE GIESE RD		STOP SIGN	N	DRY	ANGL	N/A	S	-N					000
CITY	SA	0	SE PLEASANT VIEW DR	CN	02	0		DRY	PDO	PSNGR CAR			01	DRV	NONE	00	Unk UNK	000	000	
N	3P	45	28	20	.93	-122	28												000	
																			000	
09195	N	N	N	N	08/06/2016	16	SE GIESE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT				03
NO RPT	SA	0	SE PLEASANT VIEW DR	CN	04	0		DRY	PDO	PSNGR CAR			01	DRV	NONE	00	Unk UNK	000	000	
N	3P	45	28	20	.93	-122	28												000	
																			000	
11176	N	N	N	N	09/20/2016	17	SE GIESE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT				03
NONE	TU	0	SE PLEASANT VIEW DR	CN	04	0		DRY	PDO	PSNGR CAR			01	DRV	NONE	00	Unk UNK	000	000	
N	6P	45	28	20	.93	-122	28												000	
																			000	
94329	N	N	N	N	09/20/2016	19	SE GIESE RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRGHT				02
NONE	TU	0	SE PLEASANT VIEW DR	CN	04	0		DRY	PDO	PSNGR CAR			01	DRV	NONE	00	Unk UNK	000	000	
N	5P	45	28	20	.93	-122	28												000	
																			000	

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CITY OF GRESHAM, MULTNOMAH COUNTY

RICHAY RD at 190TH DR, City of Gresham, Multnomah County, OR 97219 from 01/01/2015 to 12/31/2019
1 - 4 of 10 Crash records shown.

SER#	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	OFFRD	WTHR	CRASH	MOVE	PRTC	INJ	A	S	ACT	EVENT	CAUSE	
INVEST	E	A	B	U	I	C	O	DAY	DIST	FIRST STREET	DIRECT	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED	LOC	ERROR		
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LEGS	TRAF-(#LANES)	DRWY	LIGHT	SURVY	TO	P#	TYPE	SVRTY	E	X	RES		
UNLOCK?	D	C	S	V	L	K	LAT	LONG	LRS	LOCTN	CONTL												
15576	Y	N	N	01/03/2016	17	SE RICHAY RD	INTER	3 -LEG	N	STOP SIGN	N	ICE	REAR	UNKN	0	STRGHT					013	01	
CITY	SU	0	SE 190TH DR	W	06	0			N	DARK	INJ				01	DRVR	NONE	0	Unk UNK UNK	042, 047	000	00	
N	5P	45 28 2.73	-122 28	.09																		000	01
04047	N	N	N	05/10/2019	17	SE RICHAY RD	INTER	3 -LEG	N	STOP SIGN	N	CLR	ANGL-STP	01 NONE	9	TURN-R					08		
CITY	FR	0	SE 190TH DR	W	06	0			N	DRY	TURN	N/A		N -W							000	00	
N	9P	45 28 2.73	-122 28	.09					N	DARK	PDO				01	DRVR	NONE	0	Unk UNK UNK	000	000	00	
10090	N	N	N	09/17/2015	17	SE RICHAY RD	INTER	3 -LEG	N	STOP SIGN	N	RAIN	ANGLE-OTH	01 NONE	0	STRGHT					02		
NONE	TH	0	SE 190TH DR	CN	03	0			N	WET	ANGL	PRVTE	N -S								000	00	
N	6P	45 28 2.73	-122 28	.09					N	DAY	PDO				01	DRVR	NONE	0	F UNK UNK	028	000	02	
11752	N	N	N	10/27/2015	17	SE RICHAY RD	INTER	3 -LEG	N	STOP SIGN	N	CLR	ANGLE-OTH	01 NONE	0	STRGHT					02		
NONE	TU	0	SE 190TH DR	CN	04	0			N	DRY	TURN	PRVTE	S -N								015	00	
N	5P	45 28 2.73	-122 28	.09					N	DAY	PDO				01	DRVR	NONE	31	M OR-Y UNK UNK	000	000	00	

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CITY OF GRESHAM, MULTNOMAH COUNTY

RICHEY RD at 190TH DR, City of Gresham, Multnomah County, OR 97219 from 01/01/2015 to 12/31/2019
5 - 7 of 10 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE			
SR#			INVEST	E	A	U	I	C	O	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT SURF	COLL	OWNER	FROM	G	E	LICNS	PED				
RD DFT	B	L	G	N	H	R	TIME	FROM	TO	SECOND STREET	LOCTN	(#LANES)	DRWY	LIGHT	SVRTY	P# TYPE	SYRTY	E	X	RES	LOC	ERROR		
UNLOC?	D	C	S	V	L	K	LAT	LONG	LRS	INTER	3 -LEG	N	CLR	ANGL-OTH	01 NONE	0	TURN-L					03		
08153	N	N	N	07/14/2016	17	SE RICHEY RD	CN	SE 190TH DR	03	0	STOP SIGN	N	DRY	TURN	PRVTE	W -N	01 DRV	NONE	59	F	OR-Y OR<25	015	00	
CITY	TH	0	WE	SE 190TH DR	03	0	DAY	INJ	INJ	PRVTE	PSNGR CAR	01 NONE	0	TURN-L	W -N	01 DRV	NONE	000	000	000	000	00		
N	N	2P	45 28 2.73	-122 28	.09											02 FSNG	INJB	16	F	000	000	000	000	
02802	N	N	N	03/02/2016	17	SE RICHEY RD	INTER	3 -LEG	N	STOP SIGN	N	WET	TURN	N/A	S -N	01 DRV	NONE	00	Unk	UNK UNK	013	000	000	
NONE	WE	0	SE 190TH DR	CN	04	0	DAY	PDO	INJ	PRVTE	PSNGR CAR	02 NONE	9	TURN-L	W -N	01 DRV	NONE	00	Unk	UNK UNK	000	000	000	
N	N	UNK	45 28 2.73	-122 28	.09											02 FSNG	INJB	12	F	021	000	000	000	
04167	N	N	N	05/05/2018	17	SE RICHEY RD	INTER	3 -LEG	N	STOP SIGN	N	DRY	TURN	N/A	S -N	01 DRV	NONE	00	Unk	UNK UNK	000	000	000	
CITY	FR	0	SE 190TH DR	CN	03	0	DAY	INJ	PRVTE	PSNGR CAR	02 NONE	0	TURN-L	W -N	01 DRV	NONE	22	F	OR-Y OR<25	028,021	000	000	000	
N	N	2P	45 28 2.75	-122 28	.09											02 FSNG	INJB	12	F	000	000	000	000	
05667	N	N	N	07/02/2018	17	SE RICHEY RD	INTER	3 -LEG	N	STOP SIGN	N	DRY	TURN	UNKN	N -S	01 DRV	NONE	00	Unk	UNK UNK	028,021	000	000	000
NONE	MO	0	SE 190TH DR	CN	03	0	DAY	INJ	PRVTE	PSNGR CAR	03 NONE	0	STOP S -N	PRVTE	PSNGR CAR	01 NONE	0	75	F	OR-Y OR<25	011	000	000	000
N	N	7A	45 28 2.73	-122 28	.09											02 FSNG	INJB	16	F	000	000	000	000	

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CITY OF GRESHAM, MULTNOMAH COUNTY

URBAN NON-SYSTEM CRASH LISTING

RICHIEY RD at 190TH DR, City of Gresham, Multnomah County, 01/01/2015 to 12/31/2019

8 - 10 of 10 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A	S	FRTC	INJ	G	E	LICNS	PED	ACT	EVENT	CAUSE
SER#									DI ST	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	OWNER	FROM							LOC	ERROR			
INVEST	B	A	U	I	C	O	DAY		FROM	SECOND STREET	LOCTN	(#LANES)	CONTLN	DRWY	LIGHT	SRTY	V# TYP E	TO	P# TYPE	SRTY	E	X	RES					
RD DFT	B	L	G	N	H	R	TIME		LONG	LRS																		
UNLOCK?	D	C	S	V	L	K	LAT																					
82642	N	N	N	07/29/2018	17	SE	RICHIEY RD	INTER	3 -LEG	N	CLR	ANGL-OTH	01	NONE	9	TURN - L											0.3	
NONE	SU	0	SE	190TH DR	CN			STOP	SIGN	N	DRY	TURN	N/A		W -N												0.0	
N	9A	45	28	2.73	-122	28	.09	04	0	N	DAY	PDO		PSNGR CAR			01	DRVR	NONE	00	Unk	UNK				0.0		
N																											0.0	
08794	N	N	N	N	09/28/2018	17	SE	RICHIEY RD	INTER	3 -LEG	N	CLR	FTX OBJ	01	NONE	9	TURN - L										0.8	
CITY	FR	0	SE	190TH DR	CN			STOP	SIGN	N	DRY	FTX	N/A		W -N											0.0		
N	7P	45	28	2.73	-122	28	.09	04	0	N	DUSK	PDO		PSNGR CAR			01	DRVR	NONE	00	Unk	UNK				0.0		
N																											0.0	

MULTNOMAH COUNTY
MULTNOMAH COUNTYRICHEY RD at FOSTER RD, City of Outside City Limits, Multnomah County, 01/01/2015 to 12/31/2019
1 - 4 of 17 Crash records shown.

S	D	M	P	R	J	S	W	DATE	MILEPOST	COUNTY ROADS	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE						
SER#			INVEST	E	A	U	I	O	DAY	DIST FROM INTERSECT	RD CHAR DIRECT	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED							
RD DFT	B	L	G	N	H	R	TIME			LOCNTN	LEGS	DRWY	LIGHT	SURVY	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	
UNLOCK?	D	C	S	V	L	K	LAT			LONG	#LANES)	CONTL	DRWY	OBJ	01 NONE	0	STRGHT									
04358	Y	N	N	N	04/25/2015					SE FOSTER RD	INTER	3 -LEG	N	Y	CLD	FIX OBJ	01	PRVTE	S -N						128,062	30
COUNTY	SA	0							SE RICHEY RD	N																
N	11P								SE RICHEY RD	06	0															
N			WE	0					SE RICHEY RD	N																
N	4P								SE RICHEY RD	06	0															
N	45 27 54 .37	-122 28							SE RICHEY RD	06	0															
N			54 .4						SE RICHEY RD	06	0															
05738	N	N	N	05/31/2017					SE FOSTER RD	INTER	3 -LEG	N						CLR	S -1STOP	01	NONE	0				29
NONE									SE RICHEY RD	N																
N	WE	0							SE RICHEY RD	06	0															
N	4P								SE RICHEY RD	06	0															
N	45 27 54 .37	-122 28							SE RICHEY RD	06	0															
N			54 .4						SE RICHEY RD	06	0															
08111	N	N	N	09/08/2018					SE FOSTER RD	INTER	3 -LEG	N						CLR	S -1STOP	01	NONE	9				29
NONE									SE RICHEY RD	N																
N	SA	0							SE RICHEY RD	N																
N	WE	0							SE RICHEY RD	06	0															
N	4P								SE RICHEY RD	06	0															
N	45 27 54 .37	-122 28							SE RICHEY RD	06	0															
N			54 .4						SE RICHEY RD	06	0															
82557	N	N	N	07/11/2018					SE FOSTER RD	INTER	3 -LEG	N						CLR	S -1STOP	01	NONE	9				29
NONE									SE RICHEY RD	N																
N	WE	0							SE RICHEY RD	N																
N	4P								SE RICHEY RD	06	1															
N	45 27 54 .37	-122 28							SE RICHEY RD	06	1															
N			54 .4						SE RICHEY RD	06	1															
03934	N	N	N	04/16/2017					SE FOSTER RD	INTER	3 -LEG	N						CLR	ANGL -OTH	01	NONE	0				0.2
NO RPT									SE RICHEY RD	E																
N	SU	0							SE RICHEY RD	05	1															
N	2P								SE RICHEY RD	05	1															
N	45 27 54 .37	-122 28							SE RICHEY RD	05	1															
N			54 .4						SE RICHEY RD	05	1															
03934	N	N	N	04/16/2017					SE FOSTER RD	INTER	3 -LEG	N														
NO RPT									SE RICHEY RD	E																
N	SU	0							SE RICHEY RD	05	1															
N	2P								SE RICHEY RD	05	1															
N	45 27 54 .37	-122 28							SE RICHEY RD	05	1															
N			54 .4						SE RICHEY RD	05	1															

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MULTNOMAH COUNTY

RICHEY RD at FOSTER RD, City of Outside City Limits, Multnomah County, 01/01/2015 to 12/31/2019
5 - 9 of 17 Crash records shown.

S	D	M	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE
SER#	P	R J S W DATE	COUNTY ROADS	INT-REL LEGS	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED	
INVEST E A U I C O DAY	DIST FROM	FIRST STREET	DIRECT	TRAF- #(LANES)	DRWY	LIGHT	SURVY	P# TYPE	SURVY	E	X	RES	LOC
RD DFT B L G N H R TIME	INTERSECT	SECOND STREET	LOCTN	CONTLN				TO	STRAIGHT				
UNLOCK? D C S V L K LAT	LONG	LRS						W-E	PSNGR CAR	0.2 PSNG	INJ/C	20 F	000
82227	N	N	SE FOSTER RD	INTER	3-LLEG	N	CLR	S-1STOP	0.1 NONE	0	STRAIGHT		29
NONE	WE	0	SE RICHEY RD	E	STOP SIGN	N	DRY	REAR	PRVTE	E-W			000
N	4P	45 27 54 .37 -122 28		06	0	N	DAY	INJ	PSNGR CAR	0.1 DRVR	NONE	6 2 M	OR-Y OR<25
N	5P	45 27 54 .37 -122 28							0.2 NONE	0	STOP		026
N	5P	45 27 54 .37 -122 28							PRVTE	E-W	0.1 DRVR	INJ/C	4 6 F
06677	N	N	SE FOSTER RD	INTER	3-LLEG	N	CLR	S-1STOP	0.1 NONE	0	STRAIGHT		29
NONE	FR	0	SE RICHEY RD	E	STOP SIGN	N	DRY	REAR	PRVTE	E-W			000
N	5P	45 27 54 .37 -122 28		06	0	N	DAY	INJ	PSNGR CAR	0.1 DRVR	NONE	0 0 M	UNK OR>25
N	5P	45 27 54 .37 -122 28							0.2 NONE	0	STOP		026
N	5P	45 27 54 .37 -122 28							PRVTE	E-W	0.1 DRVR	INJ/C	5 0 M
01542	Y	N	SE FOSTER RD	INTER	3-LLEG	N	SNOW	ANGL-STP	0.1 NONE	9	TURN-R		124
NONE	WE	0	SE RICHEY RD	E	STOP SIGN	N	ICE	TURN	N/A	S-E			08 .01
N	5P	45 27 54 .37 -122 28		06	0	N	DUSK	PDO	PSNGR CAR	0.1 DRVR	NONE	0 0 UNK UNK	000
N	5P	45 27 54 .37 -122 28							0.2 NONE	9	STOP		011
N	5P	45 27 54 .37 -122 28							N/A	E-W	0.1 DRVR	NONE	0 0 UNK UNK
04752	N	N	SE FOSTER RD	INTER	3-LLEG	N	CLR	S-1STOP	0.1 NONE	9	STRAIGHT		29
NONE	TH	0	SE RICHEY RD	E	STOP SIGN	N	DRY	REAR	N/A	E-W			000
N	8A	45 27 54 .37 -122 28		06	0	N	DAY	PDO	PSNGR CAR	0.1 DRVR	NONE	0 0 UNK UNK	000
N	8A	45 27 54 .37 -122 28							0.2 NONE	9	STOP		000
N	8A	45 27 54 .37 -122 28							N/A	E-W	0.1 DRVR	NONE	0 0 UNK UNK
10418	N	N	SE FOSTER RD	INTER	3-LLEG	N	RAIN	O-1 L-TURN	0.1 NONE	9	TURN-L		02
NONE	FR	0	SE RICHEY RD	CN	UNKNOWN	N	WET	TURN	N/A	N-E			000
N	UNK	45 27 54 .37 -122 28		04	1	N	DAY	PDO	UNKNOWN	0.1 DRVR	NONE	0 0 UNK UNK	000
N	UNK	45 27 54 .37 -122 28							PSNGR CAR	0.1 DRVR	NONE	0 0 UNK UNK	000

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are represented or that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

OREGON. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

DS380
5/05/2021

JULIETTE COUNTY

RICHEY RD at FOSTER RD, City of outside City Limits, Maltzona County, 01/01/2015 to 12/31/2019
COUNTY ROAD CRASH LISTING

DISCLAIMER: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811-720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is mandatory for all drivers and police officers, the Crash Analysis and Reporting Unit does not guarantee that all qualifying crashes are represented nor can assurances be made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

MULTNOMAH COUNTY
MULTNOMAH COUNTYRICHEY RD at FOSTER RD, City of Outside City Limits, Multnomah County, 01/01/2015 to 12/31/2019
14 - 16 of 17 Crash records shown.

S	D	M	P	R	J	S	W	DATE	MILEPOST	COUNTY ROADS	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	PRTC	INJ	A	S	CAUSE						
SER#			INVEST	E	A	U	I	O	DAY	DIST FROM INTERSECT	RD CHAR DIRECT	RNDBT	SURF	COLL	FROM	TO	P#	TYPE	G	E	LICNS	PED	ACT	EVENT		
RD DFT	B	L	G	N	H	R	TIME		UNLOC?	LONG	LOCTN	(#LANES)	CONTL	DRWY	LIGHT	SVRTY	#	TYPE	SVRTY	E	X	RES	LOC	ERROR		
13563	N	N	N	11/30/2017	SE	FOSTER	RD		NO RPT	TH	0	CN	3 -LEG	N	CLR	ANGL-OTH	01	NONE	0	TURN-R				02		
					SE	RICHEY	RD				02		1	STOP	SIGN	N	DRY	TURN	PRVTE	E	-N			000	00	
					N	4P	45 27 54 .37	1.22 28						N	DAY	INJ	PSNGR	CAR	01	DRV	INJC	18	F	OR-Y	028	000
					N	4P	45 27 54 .37	1.22 28								02	NONE	0	STRGHT	S	-N			000	02	
																PRVTE	PSNGR	CAR	01	DRV	NONE	59	M	OR-Y	000	00
01072	N	N	N	02/11/2018	SE	FOSTER	RD		NO RPT	COUNTY	0	CN	3 -LEG	N	CLR	O-1 L- TURN	01	NONE	0	STRGHT				04		
					SE	RICHEY	RD				02		0	STOP	SIGN	N	DRY	TURN	PRVTE	S	-N			000	00	
					N	4P	45 27 54 .37	1.22 28						N	DAY	INJ	PSNGR	CAR	01	DRV	INJB	40	F	OR-Y	000	00
					N	4P	45 27 54 .37	1.22 28								01	NONE	0	STRGHT	S	-N			000	00	
																PRVTE	PSNGR	CAR	02	PSNG	NONE	01	F		000	00
																02	NONE	0	TURN-L							
																PRVTE	PSNGR	CAR	01	DRV	NONE	44	F	OR-Y	004 , 028	000
																								00	02	
02848	N	N	N	04/04/2019	SE	FOSTER	RD		NO RPT	N	0	CN	3 -LEG	N	CLR	O-1 L- TURN	01	NONE	0	TURN-L				02		
					SE	RICHEY	RD				04		1	STOP	SIGN	N	DRY	TURN	PRVTE	N	-E			000	00	
					N	5P	45 27 54 .37	1.22 28						N	DAY	INJ	PSNGR	CAR	01	DRV	NONE	20	F	OR-Y	028 , 004	000
					N	5P	45 27 54 .37	1.22 28								02	NONE	0	STRGHT	S	-N			000	02	
																PRVTE	PSNGR	CAR	02	PSNG	INJC	21	M		000	00
1095	N	N	N	N 11/13/2019	SE	FOSTER	RD		NO RPT	COUNTY	WE	0	CN	3 -LEG	N	CLD	O-1 L- TURN	01	NONE	0	TURN-L				02	
					SE	RICHEY	RD				04		1	STOP	SIGN	N	DRY	TURN	PRVTE	N	-E			000	00	
					N	6P	45 27 54 .37	1.22 28						N	DARK	INJ	PSNGR	CAR	01	DRV	NONE	17	F	OR-Y	028 , 004	000
					N	6P	45 27 54 .37	1.22 28								02	NONE	0	STRGHT	S	-N			000	02	
																PRVTE	PSNGR	CAR	01	DRV	NONE	22	M	OR-Y	000	00

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MULTNOMAH COUNTY

RICHIEY RD at FOSTER RD, City of Outside City Limits, Multnomah County, 01/01/2015 to 12/31/2019
17 - 17 of 17 Crash records shown.

S	D	M	MILEPTN	COUNTY ROADS	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A	S	ACT	EVENT	CAUSE	
SER#	P	R J S W	DATE	DIST FROM INTERSECT	FIRST STREET	RD CHAR DIRECT	RNDBT	SURF	COLL	FROM	FRTC	INJ	G	E	LICNS	PED
INVEST	E A U I C O	DAY	RD DFT	B L G N H R	SECOND STREET	LEGS	TRAF-	DRWY	LIGHT	TO	P# TYPE	SVRTY	E	X	RES	LOC
UNLOCK?	D C S V L K	LAT	LONG	LRS	LOCN	(#LANES)	CONTL	DRWY	SYRTY	02	NONE	STRAIGHT	S -N			
										PRVTE						
										PSNGR CAR	0.2	PSNG	INJ/C	24	M	000
																00